BOLT-BEARING STRENGTH OF WOOD AND MODIFIED WOOD

BEARING STRENGTH OF COMMERCIAL AIRCRAFT
PLYWOOD UNDER AIRCRAFT BOLTS

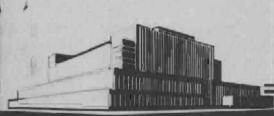
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UNITED STATES DEPARTMENT OF AGRICULTURE FOREST SERVICE

In Cooperation with the University of Wisconsin

BOLT-BEARING STRENGTH OF WOOD AND MODIFIED WOOD

Bearing Strength of Commercial Aircraft Plywood

Under Aircraft Bolts 1,2

By

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Summary

This report presents results of test performed at the Forest Products Laboratory, to determine the bolt-bearing strength and the necessary spacing of the bolt from the end and edge of members constructed of 3 species of commercially-made plywood of aircraft grade (Specification AN-NN-P-511b). The species tested were yellow birch (group I), Douglas-fir (group II), and yellow-poplar (group III). Three thicknesses of plywood of each species, 0.125 inch--three-ply (0.155 inch in the case of Douglas-fir), 0.315 inch--five-ply, and 0.590 inch--nine-ply, were tested with 3 diameters of aircraft bolt, namely, 1/4, 1/2, and 3/4 inch. Both tensile and compressive loadings were employed. Results were based upon tests of approximately 3,000 bolt-bearing specimens and 1,500 compression specimens.

This is one of a series of progress reports prepared by the Forest Products Laboratory relating to the use of wood in aircraft. Results here reported are preliminary and may be revised as additional data become available.

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This report is one of a series of reports dealing with bolt-bearing strength of wood and modified wood. Other reports are: No. 1523, "Effects of Different Methods of Drilling Bolt Holes in Wood and Plywood;" No. 1523-A, "Bolt-Bearing Strength of Laboratory Made Crossbanded Yellow Birch Compreg Under Aircraft Bolts;" and "Bearing Strength of Commercial Crossbanded Compreg Under Aircraft Bolts." Other reports will be issued as data become available.

Maintained at Madison, Wis., in cooperation with the University of Wisconsin.

The necessary end margin, measured from the center of the bolt to the end of the member, did not exceed 2.5 diameters for any grain direction or under either type of loading. Edge clearance of 1.5 diameters was adequate when the loading was parallel to the face grain and of 2 diameters when the loading was at angles of 45° or 90° to the face grain.

The bolt-bearing stresses at both proportional limit and ultimate were found to decrease with an increase in diameter of bolt and to increase with an increase in thickness of plywood within the ranges studied in this investigation. An analysis of the causes of these variations is presented in Appendix A.

The bolt-bearing stresses at proportional limit were found to bear a systematic relationship to the ultimate compressive strength, which was applicable to all three of the species tested. A suggested procedure is presented for determining bolt-bearing design stresses at proportional limit by applying this relationship to established values for ultimate compressive strength, such as are presented in ANC Bulletin 18. Design ultimate bearing stresses can safely be assumed to equal 150 percent of proportional limit stresses thus determined.

The deformations in bearing varied approximately as the thickness of plywood, but bore no consistent relationship to the diameter of bolt. The maximum deformations for the three species tested were on the order of 0.01 inch at proportional limit and 0.10 inch at ultimate.

Introduction

The efficient use of plywood reinforcing plates for bolted joints in wood aircraft structural members necessitates more accurate information than has been available concerning the bearing strength of plywood under aircraft bolts.

This study, to determine the bearing strength and the critical end margins and edge clearances for commercial aircraft plywood of different species, thicknesses, and grain orientations under single aircraft bolts of various diameters, was carried out at the Forest Products Laboratory with the guidance of the ANC Technical Subcommittee on Wood Aircraft Structures.

End margin is defined here as the dimension from the center of the bolt hole to the free end of the member and edge clearance as the dimension from the center of the bolt hole to the edge of the member measured perpendicular to the direction of the applied load.

It is believed that the data obtained will apply to other bolt sizes and plywood thicknesses within and above the range covered, except for L/D ratios at which bending of the bolt becomes a factor.

Description of Material

The plywood was of aircraft grade, manufactured commercially according to Army-Navy Aeronautical Specification AN-NN-P-511b for plywood and veneer. One species was selected from each of the three density classifications of this specification, and three thicknesses of plywood of each species were used as follows:

- Group I, rotary-cut yellow birch of 0.125-, 0.315-, and 0.590-inch nominal thickness, average specific gravity of 0.697.
- Group II, quarter-sliced Douglas-fir of 0.155-, 0.315-, and 0.590-inch nominal thickness, average specific gravity of 0.484.
- Group III, rotary-cut yellow-poplar of 0.125-, 0.315-, and 0.590-inch nominal thickness, average specific gravity of 0.475.

The Douglas-fir and yellow-poplar plywoods were each supplied by a single manufacturer, while the yellow birch was supplied by two manufacturers.

All the plywood conformed to Specification AN-NN-P=511b for quality, and all conformed in thickness except the 0.315-inch yellow birch, which was slightly undersize.

A minimum of 64 square feet of any species and thickness was available for test.

The bolts used in these tests were standard hexagon-head, steel aircraft bolts manufactured according to Army-Navy Aeronautical specification An-B-3a. The strength requirements for this class of bolts include a yield strength of 96,000 pounds per square inch in tension and an ultimate of 125,000 pounds per square inch. Sizes employed were 1/4, 1/2, and 3/4-inch nominal diameter. Specified sizes and tolerances were as follows:

1/4 inch, 0.249 (+0.0000;-0.0030) inch;

1/2 inch, 0.499 (+0.0000; -0.0035) inch;

3/4 inch, 0.749 (+0.0000;-0.0045) inch;

Matching

The variables involved in this study were: (1) Species; (2) thickness;

(3) grain orientation; (4) type of loading; (5) diameter of bolt; and

(6) edge clearance or end margin.

One combination of the first five variables defined a group of test specimens. Three groups, one for each of the three bolt diameters used. formed a series. Groups were composed of five sets of five specimens, each set having a different end margin or edge clearance. In order to determine definitely the critical distance and the bolt-bearing strength with this number of specimens, it was necessary to straddle the estimated critical dimension with the group so that at least two sets would have safe dimensions, and the loads obtained for the five sets together would indicate the point at which the increasing edge or end distance became adequate to develop the full bearing strength. From existing information, the critical dimension was estimated, and a trial group consisting of one specimen of each of five sizes straddling this dimension was tested before the complete group was cut. All specimens for one series (three groups) were cut together from a single sheet of plywood. except those of 0.315inch thick yellow birch, the panel size of which was too small for some series. Matching between the groups of a series was achieved by cutting them consecutively from the same panel.

The cutting diagram (fig. 1) shows the matching within a group. The five trail bolt-bearing specimens were cut first. Following this, the five sizes selected on the basis of results obtained with the trial specimens were cut consecutively. This pattern was repeated five times to make the group of 25 bolt-bearing specimens. The variations of end margin or edge clearance used in three different groups are illustrated in figure 2.

Compression specimens with the face grain parallel or perpendicular to the direction of loading were cut with the corresponding bolt-bearing specimens. For the bolt-bearing specimens having the face grain at 45°, however, compression specimens with both 0° and 90° grain orientation were used. Under this condition, it was not possible to alternate compression specimens with bolt-bearing specimens in the manner shown in figure 1-A for 0° and 90° specimens without being wasteful of material. The compression specimens were, therefore, cut around the edges of the panels and in groups in the interior of the panels as shown in figure 1-B.

Preparation of Specimens

The cutting and drilling of the specimens were done with considerable care to obtain true, smooth surfaces. Hollow ground saws were used because ordinary saws roughened or splintered the edges, particularly those of the Douglasfir. With these saws, careful handling of the birch was necessary to avoid burning. The speed of rotation of the saws was 3,500 revolutions per minute, and the saw diameter was 12 inches. It was possible to cut most of the specimens to within 0.01 inch of the desired dimension.

Twist drills were used for drilling the bolt holes because with proper handling of this type of drill, holes of such accurate dimension and surface smoothness could be produced that quality of hole was eliminated as a variable factor that might otherwise interfere with the evaluation of the variables under study. The drills were machine-sharpened and hand-honed and were centered accurately in the chuck. They were operated at rotational speeds of 475 revolutions per minute for the 1/4- and 1/2-inch drills and 280 revolutions per minute for the 3/4-inch drills, resulting in peripheral speeds of 311, 622, and 550 feet per minute, respectively. The rate of feed was about 1 inch per minute. The specimen was held securely in place with a hand lever during the drilling operation. The resulting holes were straight and of accurate dimension, and their surfaces had a distinct shine.

Conditioning

Material, cut in strips whose width was equal to the length of the specimen, was conditioned at 65 percent relative humidity and 70° F. for at least 1 week before it was cut into specimens and drilled. Specimens were then further conditioned several days before testing. All handling of specimens outside the conditioning room was done in closed containers. Testing was generally done in the conditioning room. Those specimens that were tested outside the conditioning room were removed one at a time, protected from moisture loss, and sampled for moisture content after test.

Method of Tests

Methods used in testing the bearing strength of plywood under bolts are classified according to the manner of application of load to the specimen as follows:

- (1) Tensile
- (2) Compressive
- (3) Modified compressive

Diagrams of the specimens and a chart indicating the extent of the testing with each method are shown in figure 3.

The same jig, adaptable to the three types of loading, was used for all tests. The main features of the jig, shown in figures 4, 5, and 6 are (1) two mild steel plates bolted to a base (one plate is removed in fig. 4), (2) spacers for adjusting the distance between the plates to accomodate the various thicknesses of plywood, (3) hardened steel bushings for adapting the jig to various sizes of bolts, and (4) a centrally-located dial gage reading in increments of 1/10,000 inch to measure displacement of the bolt.

[&]quot;Bolt-Bearing Strength of Wood and Modified Wood: Effects of different methods of drilling bolt holes in wood and plywood" Forest Products Laboratory Report No. 1523 (December 1944).

The two plates were spaced about 1/8-inch farther apart than the thickness of the specimen to provide room for extrusion of the wood fibers as the specimen was deformed under load. Bolts were used without nuts, and the heads were kept clear of the bushings. These precautions were taken to preclude the possibility of binding and of introducing friction or lateral restraint as a factor in the measured load.

Fit of the bolts in the holes in the specimens was such that they could be pushed through by hand. A projecting lug on the inner face of each bushing provided support for the bolt in the gap between the plates and the specimen without interfering with extrusion of the wood fibers. Bushings were keyed in place to insure accurate positioning and were reversible to provide support for the bolt under either tensile or compressive loading.

After placing the specimen in the jig and applying a small load to stabilize the apparatus, the dial was centered on the end of the specimen, and then positioned longitudinally to a zero reading. These two adjustments were fixed by thumb screws and could be made quickly. In the early part of the testing, contact between the plunger of the dial and the specimen was made through a flat-headed tack pressed into the specimen, but difficulties were encountered with this method, such as splitting of the thin specimens and the tack creeping out of the wood during the test. It was found that the specimen surfaces produced with the hollow ground saws used were sufficiently smooth that direct contact of the gage with the specimen produced more reliable data than could be obtained with the metal contact.

Deformation taking place in the jig, under load, between the bolt and the point at which the gage support is fixed to the jig, adds to the reading of the bolt displacement. The amount, however, as computed from the dimensions of the jig parts was inconsiderable for deformations at the proportional limit and was in all instances less than 1 percent of the deformation at maximum load

In applying load, the rate of head movement was 0.01 inch per minute as determined by a metronome and an auxiliary dial gage. A minimum of 15 readings of load and displacement were taken up to the proportional limit, and readings were continued to failure. In general, very careful attention to detail was necessary at all times to produce consistent data.

Tensile loading.--Tensile loads were applied to bolt-bearing specimens as shown in figure 5. The jig with the detachable base removed was bolted to the movable head of the testing machine. Tensile loads were applied to the specimen by means of a self-aligning Templin grip, the jaws of which covered the full width of the specimen and about 2 inches of the length, leaving a minimum of 3 inches between the grip and the center of the bolt hole.

Compressive loading. -- For compressive loading of bolt-bearing specimens, the jig was attached to the removable base and set on the bed of the testing machine as shown in figure 6. Loads were applied to the specimen with a fixed loading head. Difficulties encountered in applying bolt-bearing loads to thin materials

with a self-adjusting head were discussed in an earlier report. Lateral stability of the movable head of the testing machine also was of considerable importance in the testing of thin material. Slight shifts of the head caused uneven stress distribution at the bolt and produced erratic results. Further difficulty was experienced when the weight of the movable head was equal to or slightly less than the proportional limit load. After transfer of the weight of the head to the specimens, loading stopped until the screws of the testing machine began to take load in tension. During this interval, deformation continued. If this occurred at or near the proportional limit load, resulting irregularities in the curve tended to obscure the proportional limit.

Modified compressive loading. -- In earlier tests under tensile loading, difficulty was experienced in gripping the specimen. For this reason and also because of greater speed of the compressive method, a modification of the tensile specimen was devised by which the necessary end margin for tensile loading could be determined approximately under compressive loading (fig. 3). It was intended to test similar materials by both methods and then to proceed with the modified compressive method if the results obtained were comparable. The modified method was dropped, however, because adequate improvement was made in the tensile loading technique.

Testing technique other than the method of load application was the same as described for tensile and compressive loadings. It was necessary that the specimen be of sufficient width so that failure would not be affected by the proximity of the two loading blocks to the bolt hole.

Compression Test of Plywood

The compressive strength of the plywood was determined by the methods and with the type of lateral supporting device described in ASTM tentative specification D805-44T, paragraphs 5-9, inclusive, and figures 1 and 2.

Specimens were 1 inch wide by 4 inches long by the thickness of the material. They were supported laterally to prevent bending under load. Deformations in a 2-inch gage length were measured with a Marten's mirror compressometer reading to 1/100,000 inch. The compressometer was removed after the proportional limit was passed, and the specimen was then loaded to failure.

Loads were applied by means of a hydraulic press, the head speed of which was controlled to 0.012 inch per minute by means of a dial gage reading to 1/10,000 inch and a metronome.

[&]quot;Bolt-Bearing Strength of Wood and Modified Wood: Bolt-Bearing Strength of Laboratory-made, crossbanded, yellow birch compreg under aircraft bolts."

Forest Products Laboratory Report No. 1523-A (March 1945)

Tables and Charts

Typical load-deformation curves for specimens having adequate dimensions to produce failure in bearing under the bolt are shown in figure 7. From these curves, the proportional limits could be determined fairly accurately. In general, the loads reached definite ultimate values as shown in curve A. Those curves that did not show definite ultimates, regardless of the displacement of the bolt, either leveled off momentarily, as in curve B, or assumed a very small and uniform slope, as in curve C, at points which agreed well in both load and deformation with more definite values obtained from curves of type A. Since loads at excessive deformations would be of little significance, values thus selected were assumed to represent reliable ultimate loads.

Figure 8 shows a plot of a typical relationship between the end margin (or edge clearance) and bearing stresses at the proportional limit and at ultimate. Each plotted point is the average of five tests. No value is shown for the proportional limit of the first set of five specimens because failure occurred through the inadequate margin and at a bearing stress less than the proportional limit.

Figure 9 shows the influence of diameter of bolt on the ratio of bearing to compressive stresses for the several species and thicknesses of plywood.

Tables 1 through 5 present the summarized results of tests on those bolt-bearing specimens that had adequate dimensions to produce bearing failures and on all the compression specimens. In each table, columns 1 through 9 summarize the bolt-bearing tests and columns 10 through 17 the compression tests. Each value shown is the average for the number of specimens listed and is without adjustment. The maximum and minimum values indicate the variability. The specific gravities of the compression specimens (column 16) are based on volume at test and weight when ovendry. The specific gravities and moisture contents of the compression specimens are assumed to represent the bolt-bearing specimens also, since corresponding specimens were cut from the same panel (fig. 1) and processed and tested under the same conditions. Bearing stresses at proportional limit and ultimate are further summarized in table 6.

Variations in moisture content were so small that no adjustment of the strength values to a common moisture content was necessary. While there were variations in specific gravity within each species the same values of specific gravity were essentially common to bolt-bearing and compression specimens because of the method of taking specimen from the original panels. It is convenient, therefore, to relate the bolt-bearing strengths to the corresponding compressive strengths. These relationships are summarized as ratios in table 7.

Tables 8 and 9 consist of summaries of critical dimensions and of deformations, respectively, for various diameters of bolt and species, thicknesses, and grain orientations of plywood.

Analysis of Results

Edge Clearance and End Margin

In general, the plywoods studied required relatively small edge clearance or end margins to develop the ultimate bearing strength. An edge clearance (measured from axis of bolt) of 1.5 diameters was adequate for the three thicknesses of each of the species under either tensile or compressive loading parallel to the grain of the face plies. Douglas-fir plywood of 0.315-inch thickness, which was tested also at 45° and 90° to the direction of the face grain, did not require edge clearances in excess of two diameters for these grain orientations.

It should be kept in mind that these tests were made on plywood specimens employing only single bolts. The edge clearances thus determined are not applicable without modification to joints employing two or more bolts in line nor to plywood used as bearing plates for wood structural members.

Critical values of end margin for loading parallel to the face grain did not exceed 2.5 diameters for the three species tested. This value was also the maximum required for 0.315-inch Douglas-fir plywood loaded at 45° and 90°

There was a tendency for the required edge clearances and end margins, expressed as multiples of bolt diameter, to increase with plywood thickness and to decrease with an increase in bolt diameter. The trend, however, was not sufficiently well defined nor of sufficient magnitude to warrant consideration of other than the blanket values presented. The proportional limit load is developed at dimensions much smaller than those required for developing the ultimate bearing strength, (fig.8).

Bolt-Bearing Strength

The average bolt-bearing stresses at both proportional limit and ultimate for grain orientation of 0°, summarized in table 6, varied with the diameter of bolt and with the thickness of plywood. In general, the values decreased with increase in diameter and increased with increase in thickness, within the ranges studied in this investigation. The results of earlier tests to determine the bearing strength of wood under bolts have shown that bearing stress at proportional limit drops off rapidly as the ratio of the bearing length to diameter of bolt (L/D ratio) increases above a certain critical value. This is the L/D ratio at which bending of the bolt becomes a factor. At ratios less than the critical value no change in proportional limit stress is considered to take place. For the ultimate bearing stress, the critical

Analysis of the cases of these variations is presented in Appendix A of this report.

MACA Technical Note No. 296.

USDA Technical Bulletin No. 332.

ratio is larger, and the percentage reduction, for ratios exceeding this value, is less. In these tests the upward trends, with increase in L/D ratio indicated in both proportional limit and ultimate bearing stresses for plywood suggest that the maximum ratio employed (2.4) is less than the critical value.

With the exception of modified compressive loading, the manner of loading did not significantly influence the bearing values. The values obtained under modified compressive loading, were generally higher than those obtained by the other methods. Since this method was employed for only 1/2- and 3/4-inch bolts in 0.315- and 0.590-inch plywoods inclusion of the results with those for the other loadings, causes the trends in the average results to be somewhat irregular. Further irregularity was introduced by the difference in specific gravity between the 0.315-inch and the 0.125- and 0.590-inch yellow birch plywood. Adjustment for this difference in specific gravity, on the basis of the relationship developed for compressive strength, reduced the irregularity but did not eliminate it. Adjustment on the basis of average compressive strength likewise failed to produce a uniform relationship between bearing stress and thickness of plywood, but the improvement was greater than was obtained by adjusting on the basis of specific gravity. This suggested using the ratio of bearing stress to compressive stress as the means of presenting these results.

Bolt-bearing tests with grain orientations of 45° and 90° made only on the 0.315-inch Douglas-fir plywood, indicated that for plywood of balanced construction, the bearing stresses at both proportional limit and ultimate are essentially the same for these grain orientations as for 0°. The conclusions drawn in this report with reference to bolt-bearing strength at 0° can therefore be presumed to apply equally well at 45° and 90°.

Compressive Strength

Average compressive stresses at both proportional limit and ultimate were essentially the same for all thicknesses of plywood of a given species. In yellow birch the effect of the higher specific gravity of the 0.315-inch plywood was apparent only in the ultimate strength. No compression tests were made on any of the species at 45°, the average of compressive strengths at 0° and 90° being used for correlation with the bolt-bearing strength of 0.315 Douglas-fir plywood at 45°. The average compressive strength for this species and thickness at 90° was essentially the same as at 0°.

Relationship Between Bearing Strength and Compressive Strength

Average ratios of bearing stress to compressive stress at proportional limit for grain orientations of 0° are shown in A of table 7, and corresponding ratios at ultimate in B of table 7. Within each species, these ratios naturally follow the same general trends as the bearing stresses themselves, decreasing with increase in diameter of bolt and increasing with increase in thickness of plywood. The magnitudes of the ratios, however, differ considerably among the three species and are somewhat higher for ultimate than for proportional limit stresses. The relationship of these two ratios to diameter of bolt for the three species are shown in figures 9A and 9B.

While all curves display a downward trend with increase in diameter, they are widely separated and are not all mutually parallel.

Average ratios of proportional limit stress in bearing to ultimate stress in compression are shown in C of table 7. These ratios display the same general trends with variations in diameter and thickness, but they are much more uniform among the three species than those in figures 9A and 9B. This is demonstrated by the close proximity of the curves of figure 9C and suggest that the relationship between proportional limit stress in bearing and ultimate in compression provides the most satisfactory basis for design. Average ratios of proportional limit stress in bearing to ultimate stress in compression for the three species studied are plotted in figure 9D. All average ratios for the individual species are within +13 percent -10 percent of the corresponding combined average.

Relationship Between Proportional Limit and Ultimate in Bearing

Average ratios of proportional limit in bearing to ultimate in bearing are shown in D of table 7. In general, these ratios vary only slightly with variations in diameter and thickness and are reasonably uniform among the three species tested. If it is desired to derive values of ultimate bearing stress from empirical values of proportional limit stress, the higher ratios are the more conservative. The use of a ratio of 2/3 (approximately the average found for Douglas-fir plywood) would correspond to the ultimate factor of safety of 1.5, currently used in design of wood aircraft structures and would provide design ultimate bearing stresses that would be adequately conservative.

Determination of Bolt-Bearing Design Stresses

The results of this investigation indicate that bolt-bearing design stresses for plywood should be based on proportional limit stresses rather than ultimate stresses. At least until the results of repeated loading tests (currently under way) become available, design "limit stresses" should not exceed proportional limit stresses. It is possible that the repeated-loading tests may indicate a "yield stress" somewhat higher and more closely related to the ultimate bearing stress than the proportional limit stress determined in this investigation.

Bolt-bearing proportional limit stresses for design purposes can be determined by applying ratios presented in figure 9D of this report to ultimate compressive strength values currently available in table 2-9 of ANC Bulletin 18. If it is desired to specify design ultimate stresses, the values should not exceed 150 percent of the proportional limit stresses thus determined.

Deformation

Deformation of the plywood in bearing under the bolt ranged in individual tests up to 0.01 inch at the proportional limit and up to 0.10 inch at ultimate.

Average values were about one-half of these maximums. There was a definite increase in deformation at ultimate and a less significant increase in deformation at proportional limit with increase in thickness of plywood. There was, however, no consistent trend of deformation in relation to diameter of bolt. In general, deformations were greater in yellow birch and yellow-poplar than in Douglas-fir.

Conclusions

- (1) Edge clearances equal to 1-1/2 diameters (measured from axis of bolt) are adequate for single-bolted connections in aircraft grade plywood under either tensile or compressive loading parallel to the grain of the face plies. For loading at 45° or 90°, edge clearances of 2 diameters are adequate. End margins of 2-1/2 diameters are adequate for tensile loading at any grain orientation.
- (2) The bolt-bearing stresses at both proportional limit and ultimate tend to decrease with increase in diameter of bolt and to increase with increase in thickness of plywood.
- (3) The bolt-bearing stresses at both proportional limit and ultimate bear systematic relationships to the corresponding compressive stresses. These relationships differ markedly, however, among species.
- (4) The bolt-bearing stress at proportional limit bears a systematic relationship to ultimate compressive strength, and this relationship is reasonably uniform for the three species tested.
- (5) Bolt-bearing stresses at proportional limit provide the most satisfactory basis for determining design stresses. Bearing stresses at proportional limit can be determined by applying relationships developed in this investigation to established values for ultimate compressive strength such as are presented in ANC Bulletin 18.
- (6) Design ultimate bearing stresses should not exceed 150 percent of the proportional limit stresses thus determined.
- (7) The deformations under bolts in aircraft plywood vary approximately as the thickness of plywood but bear no consistent relationship to diameter of bolt. The maximum deformations for the three species tested were on the order of 0.01 inch at proportional limit and 0.10 inch at ultimate.

APPENDIX A

Analysis of the Causes of Variations in Bearing Strength with Variations in Diameter of Bolt and Thickness of Plywood

In the foregoing report, it was shown that bolt-bearing stresses at both proportional limit and ultimate tended to decrease with increase in diameter of bolt and to increase with increase in thickness of plywood. It follows, therefore, that corresponding bearing loads do not vary in direct proportion to the projected bearing area. This is demonstrated in figures 8 and 9, wherein it is evident that the load carried by a bolt of a given diameter at the proportional limit or at the ultimate is not directly proportional to the thickness of the material. A straight line relationship does exist for each bolt diameter, however, within the range of thicknesses used. If extended, these lines intersect the horizontal or thickness scale. It is evident that the material near the face of the member carries less than the average stress. It can be assumed that the intercept represents the thickness carrying no stress and that the balance of the thickness carries uniform stress.

For each species, straight lines drawn to fit the points plotted for each of the three bolt diameters intersect the thickness scale at virtually the same point. If thicknesses are reduced by the average value of this intercept, uniform values of computed stress are obtained for all thicknesses used with a given bolt diameter. These stress values differ, however, with the diameter, increasing as the diameter decreases.

In figures 10 and 11, it may be seen that the load carried by a bolt at the proportional limit or at the ultimate is not directly proportional to the diameter. A straight line relationship does not exist for each thickness, however, within the range of diameters used. If extended, these lines intersect the vertical or load scale indicating a supporting effect in addition to direct bearing. A supporting effect at the edges of bolts or plates has been described as the support provided by adjacent material through fibers that continue beyond the bearing area.

Considering each species separately, straight lines were drawn to fit the points for each of the three plywood thicknesses. Dividing the intercepts on the load scale by the length of edge (twice the effective thickness) produces fairly uniform values, which indicates that the supporting effect is dependent on the length of bearing only and independent of bolt diameter.

Basic bearing stress values (table 10, columns 4 and 8) were obtained by subtracting from the load the amount of the supporting effect of the edges and dividing the remaining load by the reduced area (diameter multiplied by reduced thickness).

The three factors involved in obtaining the bearing strength of plywood under bolts are, therefore: (1) the basic bearing stress; (2) the reduction in thickness which must be made to obtain the effective area on which to apply the basic stress, and (3) the support at the edges, which must be added to the load computed by the use of factors 1 and 2.

The load in pounds which will be supported by bolts in 50-50 plywood of the species used for any combination of bolt diameter and thickness within the range of sizes covered is therefore:

$$P = (T - t) \quad (DS + 2C)$$

where

T = total thickness (inch)

t = ineffective thickness (inch)

S = basic unit bolt-bearing stress (pounds per square inch)

C = support at edges (pounds per inch)

D = diameter of bolt (inch)

Loads computed by this formula (table 10, columns 5 and 9) using the values of S, C, and t determined in these tests (table 11) are in good agreement with the test values of columns 3 and 7. From columns 6 and 10 it may be seen that 94 percent of the computed ultimate load values are within ± 9 percent of the test values. Agreement is not quite so good for the proportional limit loads. This is to be expected, since these values cannot be determined with the same accuracy as ultimate values.

The loads obtained with 0.315-inch thick Douglas-fir plywood with the face grain at 45° and 90° to the direction of loading were found in these tests to be about the same as the bearing strength parallel to grain. The supporting effect of continuous fibers (edge support) produces load-scale intercepts of similar magnitude for the three directions of loading. Differences are within the range of accuracy of determination. Since only one thickness of plywood was used for grain angles of 45° and 90°, no value of ineffective thickness could be determined. No appreciable error will result, however, from assuming the value of t for Douglas-fir to be the same for the three grain angles. An increase or decrease in t of 50 percent will not produce an error in load of more than 3 percent. This cannot be assumed to be true for other species, however, on the basis of these tests.

It is recognized that, in order to apply these refinements to design, values of S, t, and C would have to be developed for each species of plywood and that the increase in efficiency thus obtained might not justify the added steps involved. It is felt, however, that the possibility of future application of the results of this supplemental analysis justifies its inclusion in this report.

Table 1 - Summary of bolt-bearing and compression tests of valior birch, sireralt grade plymood with the face grade parallel to the direction of loading

11	2	Proportional limit ;	Proportional limit ;	Proportional limit ;	Proportional limit ;	Serring	Series	2	1 1 5 1	timate :	Proportional	Ab 44 42 A4	Number of:	of: Proportional limit		dl.Laste:	Propertional limit	Meduling of	Specifical	ontent	ii Bals bearing ii Compressive iii TonortionaliUltimate	arine siya 1:Utimat	dimension dimension
The color of the	Stress Deformation:	Stress Deformation Stress	Stress Deformation Stress	Straus	Straus	Straus	Straus	Straus	Daform	et10g	Utiputa			. 4	Warmstan		Untlaste				Mindt atre	all direct	
The control of the	(3) : (4) : (5) : (6) : (4) : (8)	(3) : (4) : (5) : (6) : (7) :	(4) : (5) : (6) : (4)	1 (5) 1 (9) 1 (5)	; (b) ; (c) ;	; (b) ; (c) ;	(2)		(8)		(6)	2	(0)	(11)	(31)	(03)	(34)	(15)	(30)			(61)	(92)
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	1904 1904 1905 1904 1905	P.S.L. Both P.L.L.	प्रश्नितः प्रश्नितः	प्रश्नितः प्रश्नितः	प्रश्नितः प्रश्नितः	प्रश्नितः प्रश्नितः	Z-1-7-4.		Inch	a tell	t Trasile toat ag	2 :: : !			Terin per Agan	1		10-3		Parcen!		. in to	Dismeter
13 1,114 1,000 1,115	1. 25 Av. 3,927 0.00277 6,799 0.0156 11 Mar. 4,485 .00550 7,292 0.0200 11 Mar. 3,376 .00197 6,158 .0120	. Av. : 3.927 : 0.00277 : 6,799 :	3,927 0.00577 6,799 1 4,485 0.00550 7,292 1 3,376 0.00197 6,158 1	3,927 0.00577 6,799 1 4,485 0.00550 7,292 1 3,376 0.00197 6,158 1	3,927 0.00577 6,799 1 4,485 0.00550 7,292 1 3,376 0.00197 6,158 1	0.00577 6,799 1 .00550 . 7,292 1 .00550 . 1,292 100197 . 6,158	6,799 : 7,292 : 6,158 :		0.0156	- ee al 40 H	75.05	====		25.54 25.54	0.00603	6.927	13.62	1,247	1 969.	7.87.	1.05	1.10	1.35
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	11 15 Av. 14,777 .00385 : 9,905 : .0512 11 Max. 6,000 .00550 : 11,038 .0860 11 Min. 14,076 .00330 8,655 .0300	Av. 14,777 : .00385 : 9,905 :	4,777 : .00385 : 9,905 : 6,000 : .00550 : 11,038 : 4,076 : .00330 : 8,655	4,777 : .00385 : 9,905 : 6,000 : .00550 : 11,038 : 4,076 : .00330 : 8,655	4,777 : .00385 : 9,905 : 6,000 : .00550 : 11,038 : 4,076 : .00330 : 8,655	00385 : 9,905 : .00550 : 11,038 : .00330 : 8,655 :	9,905 : 11,038 : 8,655 :		.0512	10000	365	::::::		3,466	.00537 .00780 .00438	6.175	4.02	1,308	.782 .805 .740	9.86		1.61	1.25
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	15 Av. 3.834 .00549 8.296 .0543 1 Mar. 4,371 .00663 8,924 .0650 Mar. 3,369 .00460 7.550 .0360	Av. 3.83400549 : 6,296 : Max. 4,37100663 : 8,924 : Max. 3,369 : .00460 : 7,530 :	3,83400549 : 8,296 : 1,3710063 : 8,924 : 3,369 : .00460 . 7,530 :	3,83400549 : 8,296 : 1,3710063 : 8,924 : 3,369 : .00460 . 7,530 :	3,83400549 : 8,296 : 1,3710063 : 8,924 : 3,369 : .00460 . 7,530 :	00549 : 8,296 : .00663 : 7,530 :	8,296 : 8,296 : 7.530 :		2000	20 4 20 5	383		F)	3,407	.00535 .00620 .00450	5,460 7,935 4,935	803		1289	80 00 80 12 10 00	1.21	1.52	8.1
1, 2, 24,		Av. 14,00300473 : 5,982 :	1,00300473 .5,982 4,67500640 6,602 3,542 00260 5,628	1,00300473 .5,982 4,67500640 6,602 3,542 00260 5,628	1,00300473 .5,982 4,67500640 6,602 3,542 00260 5,628		5,608 608 608 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		0170		55.			183 183 183 183 183 183 183 183 183 183	.00620	6,240 : 7,112 : 5,675 :	3.75	1,258 1,434 1,053	678	7.89 7.89	₹	96. 1	1.08
1, 3,098 .000217 5,465 .956 1,185 .667 8.6 1,122 1,135 .667 8.6 1,122 1,135 .667 8.6 1,122 1,135 .667 8.6 1,122 1,135 .667 8.6 1,122 1,135 1,135 .667 8.6 1,122 1,135	20 Av. 4,289 .00369 7,482 .0309 .0369 .0369 .0560 .00465 .8,562 .0540 .00465 .8,562 .0540 .00250 .6,781 .0210	Av. 4,289 : .00369 : 7,482 :	1,28900369 7,482 : 5,04000485 8,582 : 3,34800250 6,781	1,28900369 7,482 : 5,04000485 8,582 : 3,34800250 6,781	.00369 : 7,482 : .00485 : 8,582 : .00250 : 6,781 :	.00369 : 7,482 : .00485 : 8,582 : .00250 : 6,781 :	7,482 : 8,582 : 6,781 :		.0309 .0540 .0210		F.\$.77	7 2 2 2 3		\$15.5 10.5 10.5 10.5 10.5 10.5 10.5 10.5	.00502		3,54	1,307	255.33	8,0,8 0,0 W	1.31	1.24	1.25
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	11 20 1 Av 1 3,722 1 .00416 1 7,216 1 .0692 1 1 .0880 1 .0880 1 .181 1 .0880 1 .181 1 .0880 1 .181 1 .0880 1 6,740 1 .0480 1	. Max. : 3,722 : .00416 : 7,216 : .00530 : 7,787 : Min. : 3,039 : .00530 : 6,740 :	3,722 : .00416 : 7,216 : 4,264 : .00530 : 7,787 : 3,039 : .00320 : 6,740 :	3,722 : .00416 : 7,216 : 4,264 : .00530 : 7,787 : 3,039 : .00320 : 6,740 :	00416 : 7,216 : .00530 : 7,787 : .00320 : 6,740 :	00416 : 7,216 : .00530 : 7,787 : .00320 : 6,740 :	7,216 : 7,787 : 6,740 :		. 0880 . 0840 . 04480		882	22222	<i>a</i>			5,465	227	1,185	649 649	9,10	EU	1.32	1:00
1. 5 1.00 6.5 1.00 7.5 1.00 6.			**	**					-	53	reneive leading	1 4 4 4	Men old			**	-		1		1 6	0.00	
12 3.129 .00565 5.698 .60 1.219 .748 9.0 1.139 1.75	; 3,481; .00238; 6,399; ; 4,232; .00282; 7,437; ; 2,804; .00200; 5,643;	Mar. 1,481 ,00236 ,6,399 ; , , , , , , , , , , , , , , , , , ,	; 3,481 ; .00238 ; 6,399 ; ; 4,322 ; .00282 ; 7,437 ; ; 2,804 ; .00200 ; 5,643 ;	; 3,481 ; .00238 ; 6,399 ; ; 4,322 ; .00282 ; 7,437 ; ; 2,804 ; .00200 ; 5,643 ;	; 3,481 ; .00238 ; 6,399 ; ; 4,322 ; .00282 ; 7,437 ; ; 2,804 ; .00200 ; 5,643 ;	. ,00238 : 6,399 :00282 : 7,437 :00200 : 5,643 :	5,6437		. 00400 . 0400 . 0130		7.5.5		··· ·· · · · · · · · · · · · · · · · ·	3,631	.00531 .00645 .00385	5,108	864	1,169	657 682 627	7.8.7	1.12	1.25	7.00
12 3,129 .00565 5,729 .757 .124 .665 9.1 .133 1.73 1.73 .155 .261 .261 .265 .261 .265 .261 .265		. Mar. : 4,595 : .00389 : 8,907 :	1,595 : .00389 : 8,907 : 5,328 : .00580 : 9,814 : 4,046 : .00272 : 8,046 : .	1,595 : .00389 : 8,907 : 5,328 : .00580 : 9,814 : 4,046 : .00272 : 8,046 : .	1,595 : .00389 : 8,907 : 5,328 : .00580 : 9,814 : 4,046 : .00272 : 8,046 : .	00389 : 8,907 : .00580 : 9,814 : .00272 : 8,046 :	8,907 : 9,814 : 8,046 :		.0511		203		 .a.	3,470 4,605 2,890	.00558	5,698 :	823	1,219	1381	0.00,80	M 1	1.56	1.3
8 3,2%2 .00544 5.569 .57 1.199 .656 8.2 1.17 1.06 2,762 .00670 6,332 .88 1.494 .870 .637 6.9 1.31 1.06 14 3,225 .00421 7.110 .68 1.59 1.162 .86 1.18 2,608 .00320 7.110 .68 1.162 .86 1.182 8.6 1.217 14 3,802 .00425 5.783 .68 1.29 .69 1.16 .673 10.4 1.2 1.06 2,632 .00455 5.785 .68 1.29 .69 1.16 .673 10.4 1.18 1.2 4,323 .00535 5.785 .88 1.29 .69 1.29 1.29 1.29 1.29 1.29 1.29 1.29 1.2		. Mar. : 4,167 : .00693 : 9,107 : .00500 : 9,725 : .0107 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 : 8,347 : .00550 :	1,167 : .00693 : 9,107 : 1,897 : 1,897 : 1,897 : 1,895 : 1,845 : 1	1,167 : .00693 : 9,107 : 1,897 : 1,897 : 1,897 : 1,895 : 1,845 : 1	1,167 : .00693 : 9,107 : 1,897 : 1,897 : 1,897 : 1,895 : 1,845 : 1	. 00693 : 9,107 : . 00900 : 9,725 :	9,107 : 9,725 : 8,347 :		.0770		3,83		· · · · · ·	3,129 : 2,612 :	.00563 .00665 .004700	5,252	10 A	1,114	. 665 . 659	0,0,8 4 17,8	1.33	1.73	1.25
14 3,225 .00421 6,324 .90 1,356 .757 8.9 1 1.58 1.17 1.17 1.17 1.17 1.17 1.17 1.17 1.1	15 Av. 3,794 .00464 5,931 .0184 11 Max : 4,437 .00370 5,494 .0220 11 Min : 3,459 .00320 5,550 .0145	Av. 3,794004ε4 . 5,931004ε4 . 5,49400370 . 6,494	3,79400484 5,931 4,437 00370 6,494 3,495 00320 5,550	3,79400484 5,931 4,437 00370 6,494 3,495 00320 5,550	00370 5,931	00370 5,931	5,931 5,494 5,550		.0184 .0220 .0145		14.25		∞	3,242 : 3,762 : 2,392 :	.00544 .00670 .00430	5-569	F-70 50	1.199	3616	6.8.8	1,17	1.06	1.00
14 3,107 .00536 5,283 .98 1,161 .673 9,2 11 1.12 1,146 1.273 .598 10.4 11 1.18 1,146 1.273 .698 10.4 11 1.18 1.146 1.273 .698 10.4 11 1.18 1.146 1.273 .698 10.4 11 1.14 1.146 1.273 .698 1.24 1.273 .698 1.24 1.273 1.298 1.24 1.258 1.25 1.298 1.24 1.258 1.258 1.25 1.28 1.28 1.28 1.28 1.28 1.28 1.28 1.28		. Av. : 4,219 : .00313 : 7,392 :	1,219 : .00313 : 7,392 : .5,468 : .00440 : 8,029 : .3,221 : .00216 : 6,769 : .	1,219 : .00313 : 7,392 : .5,468 : .00440 : 8,029 : .3,221 : .00216 : 6,769 : .	1,219 : .00313 : 7,392 : .5,468 : .00440 : 8,029 : .3,221 : .00216 : 6,769 : .	00313 : 7.392 : .00440 : 8,029 :00216 : 6,769 :	7,392 : 6,769 :		.0650		75.89.4			3,225	.00722	6.324	805	1,356	727		1.31	1.17	1.00
12 3,323 .00535 5,700 .57 1,290 .657 7.9 .98 .94 .645 .00410 .0410	15 Av. 3,481 ,00361 7,693 ,0638 Max. 3,836 ,00413 8,159 ,0856 Min. 3,190 ,00302 7,255 ,0500	Av. 3,48100361 : 7,693 : 8,159 : .00413 : 8,159 : .00413 : 7,265 : .00413 : 7,265 : .00302 : 7,265 : .00302 : 7,265 : .00302 : 7,265 : .00302 : 7,265 : .00302 : 7,265 : .00302 : 7,265 : .00302 : 7,265 : .00302 : 7,265	3,481 : .00361 : 7,693 : 3,836 : .00413 : 8,159 : .0302 : 7,255 :	3,481 : .00361 : 7,693 : 3,836 : .00413 : 8,159 : .0302 : 7,255 :	3,481 : .00361 : 7,693 : 3,836 : .00413 : 8,159 : .0302 : 7,255 :	.00361 7,69300413 8,15900302 7,265	7,693 : 7,265 :		0638		2,43	:::::		3,107 : 3,802 : 2,632 :	00536	5.283	1834	1.161	683	8.8	1.12	1.16	1.25
15 3,455 .00523 6,242 .94 1,333 -760 8.9 II 1.17 1.14 1 1.55 243 .00940 7,327 .44 1,657 .755 9.7 II 1.14 1 1.14 1 1.2851 .00940 7,336 .43 1,116 .725 8.0 1 1.20 1 1.30 1.35 1.35 1.35 1.35 1.35 1.35 1.35 1.35	15 Av. 3,258 .00394 5,347 .0146 .1046 .10535 5,718 .0146 .10535 5,718 .0180 .116 .116 .116 .116 .116 .116 .116 .11	Av. 7,25800394 : 5,347 Max. 4,04600535 : 5,718 Min. 2,73800315 : 4,645	2,25800394 . 5,347	2,25800394 . 5,347	2,25800394 . 5,347		5,347 5,718 4,645		.0146 .0180 .0110		.61 .73 .53		۰	3,323	.00535	5,700	553	1,250 1,524 936	5693	2.0.7	90,	₹. •••••	1:00
14 2.860 .00496 5.217 5.44 1.166 674 9.0 11 1.30 1.35 1.35 1.35 1.35 1.35 1.35 1.35 1.35	20 : Av. : 4,044 : Max. : 4,585 : Max. : 3,343 :	Av. 1,044 .00398 7,145 Max. 1,585 .00470 7,790 Min. 1,3,343 .00314 6,698	1,0440038 . 7,145	1,0440038 . 7,145	1,0440038 . 7,145	. 200798 : 7.145 :	7,145 7,790 6,698		.0291		45.55			3,455 : 5,243 : 2,853 :	.00523	6.242 7.327 5.336	445	1,333	250	80,80 0,40	17	7.7. 	100
	20 4v. 3,713 .00461 7,059 .0669	Av. 3,713 .00461 7,059 . Max. 4,234 .00585 7,502 . Min. 3,123 .00315 6,712	3,71300461 . 7,059	3,71300461 . 7,059	3,71300461 . 7,059	.00461 7,059 . .00585 7,502 . .00315 6,712 .	7,059 : 7,502 : 6,712 :		0609 0830 0440		r, r, z,		 .at	2,860 : 3,479 : 2,175 :	.00496 .00625 .00345	5.217 : 5.782 : 4.769 ::	483	1,166	419.	0.6.8	1.30	1.35	1.0

Nominal: Nominal	Southed ::		***********				Action Contract of the Contrac		1	***********		Contract of the last		Section of the last	STATE OF THE PARTY		Campragadasa		of moneton
meter:t	hickness:	Number of	: Value	: Proporti	dismeter: histomess::Number of: Value : Proportional limit :	. Ult	Ultimate	-	Number o.	f: Proport	tonal limit	Withatit.	THE COLUMN	Modulus of	Specialca	Woleture	1 0	01-11-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	TOT STANTE
				T SUPPLIE	Stress theformation:	Stress	Deformation	Vitionts :	11	Skreez	Daformation:		0 2		7000000	Contents	limit stress; stress	ss: stress	n ba
(a)	(2)	1 (3)	3	(6)	(9)	(1)	(3)	(6)	(01)	(11)	(3.2)	(03)	(1/1)	(35)	(16)	(11)	(18)	(19)	(02)
पुरुष	प्रा	. 00 .0 00		4.6.7	Inch	Park	4	Texastle lond	Part - North	Eurite (Tu	inch 1987	1		P. 8. 1.		Percent			Diameter
	0.125	50	Max.	3,528 : 4,130 : 2,810 :	0.00239	6.227 7.300 5.478	0.017		Z	3.782 3.782	0.03498	5,960 6,574 4,736	78	1,201	681	8.1.8.7.7	0.93	₹ 5: 1:	2.50
	.315	8	Max.	5,463	.00520	9,676	2060.	484	17	322	.00367 .00650 .00218	6,117 6,819 1	223	1,288	735	8.7 5.0 5.1	1.37	: 1.42 :	
1/4 :		15	Av. :	4,281 : 7,556 : 3,867 :	00543	8.713 9.069 8.230	2070. 20880.	244	3	3,278	.03720	5,542 5,817 5,246	95.00	1,230	6756	9,9,8	1.31	1.57	8.50
1/2	.125	8	Kar.	3,546	.00301	5,669 1	2010.	9 4%	8	1825	00500	5,833 6,429 4,957	47.00	1,083	657	* 6.00 - 100	66	97	2.00
1/2 :	.315		Av. :	4,691 : 5,478 :	00180 00180 00290	9,459	0380	2.4.2	16	2.98	-00372 -00865 -0197	6,564	E \$541	1,414	352	2.5.5	1.7	1.25	2.00
1/2	965.	9	Av. : Max. : Min. :	3,860 : 4,108 : 3,515 :	00316	7,303	.0550	2023	20	3,188 3,507 2,627	000525	5,894	\$ & X	1,219	636	8 9 8 0 4 0	1.21	1.3	5.00
3/4	.125	85	Max.	3,679	.00327	5,955	1 0200.	325	N	7,691 2,691	.0.928	6.165 : 7.076 : 5,210 :	885	1.105	.670	4.06.7	1.00	16. :	1.50
3/4 :	315		Mox.	4,182 4,958 3,652	.00309	8,205 6,522	4120.	¥88	100	3,314 1,148 2,961	00367	6,276 i 7,467 + 5,023	£ 60.7	2,017	.754 .886 .705	הבא הלה א	1.26	1.18	5.00
3/4	. 590	01	Av. : Hax. :	3,453	.00314	6,569	9990	with the	31	3,270	.00537 .01755 .00142	5,440	3.24	1,230	. 658 1. 694 1. 696	6,6%	3.06	1.21	1.75
•			**				Mactria	d compressive	londing	- End mare	dn (90)			. 0		. :			
. .	.315	a	Av. Max.	5,475	.00639 .00780 .00545	9,580	.0580	12.00 02	ğ	3,76	0.302	6,639 7,020 6,118	28.00 03	1,404	747	4 1-10	3,46	1. 1.	:
†/r		8	Max.	5,090 5,090 5,090	.00923	9,727	.0896 .1140 .0540	18 54 Cd	ន្ទ	4,322 4,840 3,780	00311	6,432 6,930 5,810	50.46	1,569	676	408.7	1.36	1.51	:
1/2	315	9	Kin.	25.33 25.33	.00700.	7.726	.027705300160	2000	2	3,505	.00003 .00503	6,686 7,502 5,610	5.43	1,872	262	7.88.	1.38	1.16	:
1/2		Э	Av. :	2000 2000 2000 2000 2000 2000 2000 200	.00930	8,291 7,700	4500 0940	2000	2	4,134 4,830 3,510	.0:1629 .0:1715 .0:1565	6,369 1 6,780 5,790 1	354	1,320	169.	-6.9	1.3 8.1	1.30	:
3/4 ::	.315	9	Max.	5,54	.00612	5.520	0280°	7.69	<u>a</u>	3.550	.00522	6.709 : 7.320 : 5.990 :	24	1,466	7.566	2.65	1.37	1,13	*
3/4	964.	9	Max. :	5,130 :	.00683	7,461	0880	395	•01	4 05h	.00880	7,020	433	1,343	9000	8.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2	1.15	71:1	:

Table 2. Suresty of hot-benefits and compression tests of Nousben-Str. Street, ender plymost with the face grain namedial as the direction of leading

Column C	明	Monitoal: Monitoel:				Bolt bearing	Sela						Coa	Coopression				Solt 3	Solt hearing	1 Crit	Critical
The control of the	5,8	THEES: No	mber of:	Value :	Proportio	nasl limit :	U. t.	imate		: Number of		ional limit	:Ultimate:		: Kodulus of	Specifica	Moistura	ardec:	OASTO	newip:	19101
1		10 10	oecimens:		Stress 11	Deformation:	Stress	Deformation:	Ultimate	. Apoctaon	Stress	Peformation:			elseticity:	gravityA:	content	" Itali skr	alifitas.		
1,	2		(3)	(†	(5)	(9)	(7)	(8)	(6)	(20)	(17)	[12]	(13)	(14)	(15)	(16)	(11)	(96)		ļ.,	(0
	III	я.			युक्त	Lach	17-14	प्रधा	Tenalle		A &	inch par inch reace (TC)	1		94 K		Pursent			Diame	to to
1,	0.1	ئ • • • • • •	15 :	Av. Max.	2,801 3,467 2,413	0.00320	4,341 4,667 3,920	0.0113			3,026	0.00508	202° 7 7 000° 7	0.74 .81 .65	1,247 1,372 1,085	.512	5.6 7.0 7.0 7.0	載 ら	ð. 		8
1,	₩.		8	Av. : Max. : Min. :	2,790 : 2,308 :	.00520	4,303 : 4,697 : 3,884 :	.0212	R. S. A.	4	1,207	001967 001960	4,585 4,949 1,164	.69 .77 .53	1,102	\$8,9°	6.7	18	n n n n		KG
1,	5	 &	15 :	Av. Esx.	2,702 : 3,135 : 2,410 :	.00556	4,692 : 5,010 : 4,219 :	.0573	4000	4	3,346 + 3,723 + 2,727	049900	4,691 :	.76 44 56	1,073		285	8	7		ะ
15 Mar. 2, 670 Control 1,750 Control	-	55	8	Max.	2,315 : 1,600 : :	.00200	3,663 : 4,130 : 3,216 :	. 0220	28.53	7	3,455	.00536	1,649 : 5,162 : 4,000 :	.74 .63	1,294	.502 :521 :476	8.9.8 5.0.7	£9.	£.		8
15 Mar. 1,000	4	15	 K	Av. Max.	2,433 : 2,820 : 2,032 :	.00268 .00475 .00105	3,401	0115	36%	2	3,346	.00566 .00637 .00435	4,237 : 4,767 : 3,938 :	9.7.9	1,048 : 1,249 : 893 :	38.95.3	2000	ු මු 	- 8, 		8
15 Mar. 3,502 0.00775 4,511 0.027 1.75 1.25 3,130 0.00574 4,037 1.75 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.206 1.311 1.306 1.307	ŗ,	 &	15	Max.	2,711 : 3,003 : 2,427 :	.00350	4,186 : 4,361 : 4,047 :	.0165	chi çò		2,1630	00590 44700 60386	4, 610 :	.68	1,096	2 5 5 4 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	7.8	40 30			8
15 Mr. 3,521 0.00566 4,130 0.0121 0.056 1,131 0.0559 1,131 0.0569 1,132 0.0569 1	7	55 :	15 :	Av. : Max. : Min. :	3,201 ; 3,062 ; 2,749 ;	.00503	4,511 : 5,020 : 3,760 :	.0127 .0180 .0085				.00534 .00687 .00352	1,037 : 5,163 : 2,968 :	26.5	1,206 ::	.526 :	7.80 2.00	9:1	11.12		<i>\</i> Ω
15 Av. 3;521 .00674 4,844 .0282	10	15 ::	15 :	Av. :	2,777 : 3,007 : 2,517 :	.00406	14,100 :	1210.	. 68 . 76	£7	3,3,657	.00537	4,351 : 4,980 : 3.720 :	.66 .76 .58	1,075	. 4571 . 4567	10.8	. 97	<u></u>		8
15 Av. 2,700 .00263 3,948 .0030 .66	5	 &	12	Av. s Max. s Min. s	3.582 : 3.774 : 3.093 :	. 00800.	4,844 5,229 4,536	02420	27.	21	3,434	.00800	4,719 : 4,975 : 4,216 :	.83	1,034	34.25	7.87	1.03	1.03		8
25 Mv. 2,621 .00330 3,748 .0094 .70 13 3,065 .00574 4,370 .68 1,066 .475 7.7 :: .86 .86 .86 .87 .88 .88 .88 .89 .00480 4,676 .0120 .81 .81 .92 .99 1,142 .492 .8.6 : .86 .88 .89 .804 .89 .84 .85 .89 .89 .84 .85 .89 .89 .84 .85 .89 .89 .84 .89 .89 .84 .89 .89 .89 .89 .89 .89 .89 .89 .89 .89	4		15 :	Av. Max.	2,700 : 3,115 : 2,240 :	.00263	3,948 : 4,206 : 3,551 :	0000.	89.	4	3,618	.00710	, 1728 : 5,065 : 4,445 :	.76 .69	1,293		8.9.8 0.10	75			8
20 Av. 2,772 .00492 4,395 .0178 .63 14 3,508 .00731 4,716 .74 977 .468 7.7 : 168 7.7 : 179 .99 84 1.225 .480 5.0 i .09 95 .480 5.0 i .00 95 9.0 i .00 9	i.		25	Av. 1 Max. 1	2,997 :	.00350	3,748	4600.	07.	£1	3,063	04400°	3,840	.53 .53	1,066	1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1	5.6		98.		. 8
20 Mr. 2, 457 .00423 3,728 .0123 .66 1 15 3,340 .00583 4,648 .71 1,157 .504 8.7 1: .74 .80 Min. 2, 052 .00780 .0120 .77 .57 1: .74 .80 Min. 2, 150 .00456 3,541 .0120 .0120 .77 .27 1: .74 .80 Min. 2, 150 .00456 3,541 .0120 .	10.	.	8	Av.	2,772 : 3,464 : 2,328 :	.00432	4,395 4,670 4,084	.0178 .0310 .0130	25.	∄ •	3,508 : 3,987 : 3,053 :	.00731 .00950 .00640	4,366 ::	72. 18.	977 1,225 1729	891- 084-	7.7	67.			8
15 Av. 2,560 .00436 3.541 .0130 .67 1 13 2.931 .00583 4,249 .68 1,017 .477 7.9 11 .83 .83 .83 .83 .83 .83 .83 .83 .83 .83	7		8	Av. i Max. :	2,457 : 3,032 : 2,170 :	.00780	3,728	.0123	99.		3,342	.00583	4,648 : 5,105 : 4,175 :	17:	1,157	10,524	8.88 1-04	₹			8
20 Av. 2,595 .00497 4,169 .0233 .62 .15 3,487 .00657 4,728 .74 1.067 .463 7.875 .88 1 1.067 .0350 4,368 .0350 .68 3,740 .00770 4,967 .78 1,216 .477 8.247 8.2 47 8.2 47 8.2 47 8.2 47 8.2 47 8.2 47 8.2 47 8.2 47 8.2 47 8.2 47	₩.			Mex.	2,360,2	.005500 .005500	3,755	0110	55.7.99	57	2,931	.00583 .00725 .00446	7,590 3,680 3,680	848	1,017	123	7.9				8
	ċ		8	Av. : Mex. : Min. :	2,595 : 2,418 :	24400	4,169 : 4,368 : 3,938 :	.0233	36.50	15	3,487	.00770 .00770	4,728 : 4,967 :	47.	1,067	515	7.8		80		8

11	opandia :				Bolt bear	91116	The family .		- Simber	of Pe	opertional	Limit .U	ltimeter: J	. Fronortional	to sulphysical	Srechte	Part are se	Compre	Compressive	idimeaming -:
Bter:t}	nickness:	<pre>dismeter:thickness:Number of: Value</pre>		OT SEED TO	formation	Stress	mation:	rational and a state of the sta	Tipedia.	DD4:	rate that	TEALLISE	etrese:	limit Ultimate	seingiteity:gravityi.content ::Proportional:Ultimate:	gravity	Content	::Proportional:	al: Citimato	2
3	(2)	(3)	€	(5)	(9)	(2)	(8)	(6)	(10)		(11)	(21)	(13)	(14)	(15)	(16)	(21)	: (18)	(13)	(S)
Inch	dod!			i i	Inch		Inch		7 7 7 7	4	4	b)	P. 6. 1.	51	F. 8. 1. x10-3		Ferrant			Dispelation in the second
1/4 :	0.155 :	8	Av.	2,520 : 3,133 :	4/200.0	4,055 :	0.0118 :	0,62	11s lendi		109 : 770 :	C,0004004	, 308 ; 990 ;	0.71	1,177	0.486	F 19	0.61	#6.0 11.	8
1/4	.315	15	Min.	2,829	.00130	3,640 :: 4,318 :: 4,760 ::	. 4700.	2 24	2		er, er, er (er,	.00416 .00700.	3,660 : 4,673 : 5,110 :		327 1.098 1.280	£ 50.	9 20			2.50
1/4	.590	8	Min. Av.	2,660 :	.00520	3,970	0600 0600 0600 0600	si siei				0,00500	4,322 4,5601	5. E.E.	11 15 E	25 10 11	at 141010	50	1.03	
1/2 :	.155	15	Min.	2,522	.00220	4,1(3)	00900	k 2.82	 			-003E)	5,090	55.5	1,289	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	8.0.7	80 80	 8.	
1/2	.315	8	Av. Max.	2,791 : 2,655 : 2,132 :	.00381	4,059 4,960 3,425	.0140	86.	# ####			.00380 .00582 .00582	4,702 : 5,120 : 4,374 :	.65	1,147 1,359 1,359	\$87	200		98.	5.00
1/2 :	.590	 ₹	Av.	2,535 : 2,610 : 2,149 :	.00283	3,744	.0224 .0520	25.2	2 1111		3.632	00614 00675	4,410 :	25.35	1,023 1,117 974	1468 179 179				1.50
3/4 :	.155	8	Av. Max.	2.396 : 3.189 : 1.771 :	.00355	3.140 :	.0119 :	1000	9 11:11		3,238 1	.005173 .00617 .00213	5,435	47.583	1,300	基件 基	5.89	47.		5.00
3/4 :	.315	61	Max.	2,457 : 2,818 : 2,090 :	.00419	3,868 4,155 3,410	01.00 01.02 00.000	525	2 2222		48 48 48 48 48 48 48 48 48 48 48 48 48 4	60,000 100,000 100,000	4,705 5,195 4,020	. 57 94 99 99	1,264 1,1841 1,044	724 812 754 755	4.7		. 82	8
3/4 :	.612	15	Av. Max.	2,646 2,646 2,080	.00289	3,642 3,642	.0084	5.					** ** ** **		-a ou us -0 o			92.2	S . 89	
••	-						••	Modified c	OMNTHERIT	ibed a	ne - End m	mirato (W)							- 1	٠,
1,44 ::	.315 :	29	Av. Bax. Min.	3,020 3,3 ⁴² 2,790	.00550	4,606 5,310 4,222	.0180	25.56	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7		25,25,00	+ 96200 + 98600 + 98600	4,197 4,960 4,038	99.65	1,135	583	7.7	.97	1.05	8.2
	. 590	35	Av. Hex.	3,366	.00866	5,255	.0304 .0680 .0150	13.	8 := : : :		3,945	003700	4,814 5,210 4,094	.63	1,263	±233	7.8 2.5 4.5	1.02	1.01	2,0
1,12	.315	8	Mex.	2,52	.00563 .00480	3,932	.0184 : .0250 :	3.50	ส ====		38.58 38.58	.005553 .002599	4,295 : 4,760 : 3,752 :	ఫ్రజ్ర	1,189 2,051 834		7.8	.83		1.50
1/2 ::	. 590	8	Mex.		.00674 .00810 .00580	1,426 1,426 1,076	.0310	189.	====			22500 22500 22500	4,849 : 5,193 : 4,635 :	69. 55.	1,059 1,204 690	24. 1964. 1657.	7.8	£,	e.	2.2
 	.315	8	Av. : Max. : Min.		.00616 .00490	1,068 : 4,360 : 3,680 :	. 0170 . 0270 . 0120	क्षेड <u>्</u>	8		3,069	.00759 .00750 .00490	4,165 5.058 4,196	18.	1,115	30.4	2.5		16.	5.00
 4	. 599	8	Mex.	3,284 3,750 2,839	.00719 .00840	4,774	.0250	5. EF.	5 ====:		3,366 :	. 00636 . 00700.	4.0.4 4.5.4	69, 48,	1,065	. 482 508 470	7.7	8		8.8

Publ 5 - Parager of high-booling and desired to the care of the care to the few to the desired to the care of the

ori theat			(36)	Dipertera	2.00	1.50		2,00	R 2	8		2,5	2.00	R
		1	(88)		1.13	<u>e</u>		1.01	3	ŧ		1.08	8,	8
Sold bearing	1.3	rilett strass stress	(24)		0.97	92.		86,	48,	-77		66 *	₩.	¥
1	of ature:		(62)	Parenas II	252	7.9	3	0,000	4.60	0.00	Ħ	7-1	1.50	, r.
	Spindiffer slick etures:		(25)		201	PAP		283	384	483		325	12.5	823
Section of the Contract of		Redelin of	(21)	1447 1447	946 1,116 940 840	1,068	5 53	1,037 1,081 985	1,126	1,126	ad .	1,066	1,043	1,074
24,447775544	g	Toporkionalis Making	(30)	90e v.,	39.0	.68		99.	, 68 , 72 , 63	.58 .76 .59		27.	. 69	86.
11000000000	Perpendidules -to-grain	1 Military I	6.9	Death	4,042 4,436 3,634	1989		\$ 35 E	122	5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5. 5	7	633	4.500 s	999
	Perpendid	of organization	(97)	20 C.	9:00565 101700.	000500	1	00029	00/175	.00e02 .00779 .00498		45,000 45,000 100,400	12500	19500
Compression		Proportional light a	(11)	Trans	2,756 1 3,145 1	2,918 : 3,530 4 2.380 7		3,037	2,811 3,135 # 2,397	2,736 1		3,300 = 2,500	2,968	3,920
		franks of	(PE)		OI.	9		ø	ed .	3		4	180	2
		identes of funbar of a statement of the	Det .	14	1,190	452,4 455,4	(A)	1,370	1,190 +	1.370	300 1 (30)	987	27 T	1,600
***************************************	rafa	a le	(40)	See clearwing	0.70	89. 49.	Mary minher	79.	. 7. 87. 49.	.67 - 78 - 55	Bee - Zerl	.99	79.	30.74
	o-grafa	T TUSTERATED	033		1,054 1,054 1,840 3,840	4,539 £ 5,086 ± 3,854 ±	and the same	4, 160 3, 450 3, 450	4,424 4,890 3,871	4,415 4,931 3,871	stva load	4,805 : 5,340 : 4,420 :	4,823 : 5,580 : 1,420 :	4.08 5.00 5.00 5.00 5.00 5.00 5.00 5.00 5
	Parallel-to-grain	Proportional Limit at	020	Joseph 1 Pre-Property Committee Josephson -	0.60576 .00730 .00395	96.00 96.00	CONTRACTOR OF THE PERSON	.00529	.00541	.00526 .00593 .00446	Modified compressive load	06,400	.00523	00520
of Real Property lies		Proportion Stream the	(E	1000	588	2000	or .	134 A	3,995	25.05 400 400 400 400 400 400 400 400 400 4	Mod1:	3,122	3,430	3,185
		Munber of: Pa	000		A	******		9	4	2	55 U	2 2	5	a)
= 1	Preparations	Aberra more	(6)	1 1 1 1	.52	\$4'8'		29.	.57	884		.52	19.1.29	8.18.
ring		Cornellen	(8)	सुवा	0.0270	.0221 .0420 .0130	9	.0330	.0220 .0130 :	.0300 :	5 A	.0295 : .0440 : .0187 :	.0221 : .0360 : .0130 :	.0370
اود	Ultimate	String : Def	(£)	Part .	7, 564 5,090 1,281	3,536 : 3,809 : 3,297 :	8	4,371 : 4,987 : 4,128 :	3,454:	3,629 :		5.630	3,726	, 5, 7, 1, 2, 8, 8, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,
Bolt bearing	dispeter; thickness; Number of: Value : Froportional limit :	farmes Delymanica; Serves Deformation	(9)	100 100 100 100 100 100 100 100 100 100	06500 06500	72,00		2000 2000 2000 2000 2000 2000 2000 200	00381 00400 00315	73500		. 26,600 . 00,890	00555 00200 00400	20100. 000000
- Francisco III	Proporties	Shrake 1D	(6)	Trup.	25.55 25.55	2,5272		2,923	2,845	2,192		3,5,5	3,470	522
	fr Value		(#)		Av.	May 1		Mex.	Max.	Max.		Mar.	Max.	Hex.
	s:Number of	and	(3)		1. 15	15		9		1 15		8	8	S2
Nominal: Nominal:	thicker		(2)	सुरुष्टा	0.315	.315		.315	.315	.315		.315	.315	. 315
Nominal bolt	dlameter		(3)	Inch Inch	1/7	1/2		1/1	1/5	3/4		1/1	1/5	₹

Aralman of upsetfic gravity and solvious managements for the O" and Mo" compression symmetrans. Rassed on weight when oven dry and volume at test.

Table A . - Summary of bolt-bearing and communeative tests of five plv. 0.315 inch third and the the fire and a straight the face aveing throughout to the direction of londing

16 P	Healast Hostest: balt : alvmood :-	1			Bolt bearing	ring			*			Ö	Compression			1		Bolt bearing	. Cr	Critical
1.2	(Cmass:)	Number of:	Value	Proporti Stress :	llameter liktumess Number of: Value : Proportional limit : : speciaess : : speciaess : Stress : Brotomation : Stre	Stre	Ultimate :Perormation:	5 5	portional::dumber of; Proportional limit ::specimens:: tilinite :: Street	Proporti	Projectional Hait	Ultinateiri	Destina Unitable	: Modulus : slastici	<pre>idoulus of:Specific:Motetare:</pre>	Moisture	::ProportionaliUltimata	COALIVITY	92.0	
1	(2)	(3)	(†)	(5)	1	(7)	1	(6)	(61)	â	(12)	(13)	(14)	(15)	(16)	(17)	(15)	(61)	1	(80)
L S	Inch			P. s. t.		1	क्ष			नगर	dod.	7		7.10-3		Person			al	Diametere
47	0.315	9	Av Max Min	2,599 : 3,077 : 1,988 :	0.00409 .00495 .00320	4,551 4,860 3,969	0.0370 .0760.	1900110 LOADING	Milloud .	2,872 0.006 2,555 0.006 2,112 0.009	0.00500.	1, 1084 : 1,660 : 3,550 :	0.69 .73	1.074	0.477	20018	8.6	1.1	** ** ** **	2.00
	.315	15	Max.	2,330 2,630 2,005	.00450	3,607	.0202	25.54	51	5.173	62908 59808 94400	4,173 : 4,632 : 3,898 :	.66 .73 .57	1,090 1730		7.7	***************************************	%, 		1.25
	.315	 R	Max.	2,841 : 3,562 : 2,273 :	.00386 : .00482 :	4,302 :: 4,974 :: 3,961 ::	.0250	30201040144 Loadlac 365 :: 14 375 :: 14	4	1 2,829 , 0060 2 3,396 ; 0060 1 2,032 ; 0040	, 006/15 , 006/15	4,109 : 4,450 : 3,205 :	.69 .79 .45	1,061 1,061	824 464	6.66	18	1.05		1,50
	.315	15	Kin.	2,649 : 3,025 : 2,235 :	.00341 .00790	3,620	.0102	69	çı ;	27.2	000115	4,230 : 4,709 : 3,981 :	.72 849 199	. 951 : 1,045 : 818		7.9				1.00
	+315	15 ::	Av. i Max. i Min.	2,517 : 2,975 : 1,918 :	.00411	3,629	.01%2	. 565	1	3.065	6,1500 6,1700 1,005 1,005	4,259 4,787 3,780	.72 .79 .66	978 1,127 878	502 502 	7.8.7.	90	8. 		1.00
- 200	315	8	Av. : Max. : Min. :	3.057 : 2.168 :	. 00400.	4,880 5,668 4,488	. 0379 . 0800 . 0710	29. 77.	25	3,625 2,152	00290	4,338 : 4,669 : 3,811 :	99.88. 84.	1.363		# K N #	1.01	21.1.2		2.50
	.315	18	Av.	3,088 : 3,614 : 2,692 :	. 00600.	4,256 : 4,647 : 3,862 :	.0202	5.59	8	2.62	.00634 20110. 2010.	4,438 4,896 3,968	223	: 1,018 : 1,196 : 561	535	7-7	66			1.50
	£	₹,	Av. Max.	2,781	.00621	4,100 4,575 3,562	.0280	82.79	18	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	007565	4,319 :	.573	: 1,001 : 1,202 : 768	512 512 512	- 23 T	· · · · · ·	e		1.50

Tuble 5 - Smears of bolk brother med species relies to grante to the decrease of the last of the last

The column The	boltal	Nontral :		- manual management	The name of Party	Bolt be.	bearing						ā	Couprassion					Note bearing	Critical
1	han ter	tu fokuesii	to repeated	Telup	Proporti				Though Michael	Humber	oft Propor	Money Made	Ullimate:	Proportional;	Yodelus of	Specific.	Vol. tur	diam's	98100	organica.
Color					Strass			mas then	GLASHARO 2	12,000,000	Stress	Deformation	1 100000	Ultimbe :	Stanticity.	Srevi by	Content	::Proportio	Delta Library	N 14
1. 1. 1. 1. 1. 1. 1. 1.	(E)	(2)	3	(H)	(5)	(9)	6	(8)	(6)	(07)	(11)	(42)	1 (6.9)	(34)	(45)	(91)	(63)		1	(8)
	Incib	Lach			ন্নস্থ	प्रवर्ग	Fred	Tec	Lenal				सुनान्द्र		210-12 210-13		Director	40 40 40		107-101
	*/1	3.125	8	Ar. Mar.	2000 2000 2000 2000		4,116 4,668 3,398	0.0163	55.	<u> </u>	1,253	0.00577	1,026 1,637 3,054	2.24	878 999 723	5531	7.8	1.01	1,02	1.25
1	1/4	.315	8	Av. Max. Max.	2,516	14600. 56600.	3,532	.0530	423	5	2,200	1 .00639 1 .00845	25.5	85%	702 883 488	£524 8524	8.1	1:00	1,29	1 1.25
1.5 1.5			8	Mar. Min.	2,581 5,116 2,171	07700. 07700.	25.45	. 00690 . 0300	6.83	2	2,296 1,728	1 .00524	3,080	79.	882 1,021 702	. 498	8 0.0	777	17.29	E
1			8	Mar. Mar. Mar.	2.8 3.13 1.13	. 00236 . 00232	3,4527	.0109	55.12	я 	2,356	39400.	3,802;	325	198 198 198	464.	9.5		0.95	001
13 14 15 15 15 15 15 15 15						00233	3,765	.0532	300	A	2,354	00588	3,467	10 C 10	804 886 731	. 456 478 438	3.6	%	1.09	7 7 700
14 1.15 16 that 1.25 10 10 10 10 10 10 10 10 10 10 10 10 10			5	Max.		\$2000 \$2000	3,706	.0650	2 4 4 6 5 4 6 5 6 4 6 6 6 6 6 6 6 6 6 6 6	de		400		7027	848 979 661	1.053.3 1.053.3 1.04.1	80 07 80 07 12 101	50 50 7	7,36	7.00
14	1.11	.125	9			.00207 .00202	3,680 :: 3,440 ::	.01710.	ఖ్యేట్లీ 	7	H H W W	.00564	3,363	8981	1,005	. 598-	0.68	1.03	7.09	1.25
1,12	*,	.315	æ	Ar. 1 Max. 1 Hin. 1	3,006	00369	7, 505.7 7, 305.7 7, 305.7	.0262	25.2	NO	2,190 2,535 1,898	.00500	3,221	56.55	757 894 662	1994	3.5.	1.06	Fig.	1.25
1/2	1/1	.590	8	Av. 1 Ker. 1 Min. 2	2,888	02500 04700	5.03	.0408 .0600 .0260	20 10 to	% 	2,414 2,802 1,896	.00515 .00594 .00417	3,893 4,188 3,602	172	935 1,010 846	38.5°	8,88	90-7	1.13	1.25
13.5 13 14 14 14 15 15 15 15 15	1/2	.125	8	Ar. Hax. Philosophics of the control	2,354	00338	3,011	.0136	1986	\$	2,943	00593	3,538 : 4,232 : 3,016 :	199.	823 1,025 630	385	8.6.7 4.4.5	46.	£6.	7.00
129 20 14 1.854	1/2	.315	a a	Av. 1 Max. 1 Max. 1	2,282	005500	1952	.0342	75.75	5	2,231 2,503 1,879	.00635		13.75	716 905 559	15 m 2	CI BG BG		1,08	1,00
129 20 km. 2.390 00481 3,444 0148 .56 114 2.366 00571 5,692 .64 836 .465 7.7 11 .97 .93 .82 .82 .82 .82 .82 .82 .82 .82 .82 .82	1/2	.590	51	Max.	2,151 :	06,500	3,498	.0361	X4.8	ge.	2,630 3,230 2,212	29900.		255.	1,012	£55.3	8.7			7.80
3.15 15 14 14 1 2.036 1.030 1.030 1.03 1.01 1.03 1.00 1.00 1.	3/4 2	.129	8	Av. Kax. :	2,290 :	04500 04500	2.362	.0148	38.	4	2,366	.00700	3,692 3,300	75.5	836 1,035	537	7.7	.97	5	3:1
750 15 18 18 18 18 18 18 18 18 18 18 18 18 18	3/8			Ar. I Kar. I		06550	26.9	.0303 .0360 .0240	22.5	4	2.042 2.361 1.656	9500. 007400.	3.282 ; 3.585 ; 2.773 ;	92.5	733 4567	9724	8.1	1.01	1.03	1.00
		.590		Mar.	2,305 : 2,551 : 2,115 :	00005 00005 00005	Selection of the select	.0475	19.	2	2.559 2.224 2.324	00000	3.795	âki	1,035	655	20 6 20 6 20 6	\$ =====	ê #	

: Nominal: Nominal	Nominal :			5	Bolt bear	oaring		40 00 0	11 74			III S	Сапргочатов		S. of Box	# ## ##	Bolt bearing		; ; Critical ;dimmondon
bolt :	plywood !-	Number of:	Value	bolt : plymoud	uel limit :	E C	Ultimate	onel	bimber of		Proportional limit filtimeter Proportional; Madmin of: Specific Motators:	In the term	Toportionals	:Modulum of:Specific:Moistain	Specifical	Colstars:	E	: In Elmate	
		: specimens:	- **	Stress :	Stress Deformation:	Strees : E	.Deformation:	Ul timeto	STACTONIZA	Stress	Deformation	4	3	Ca case and care			Loit stream; stream	SETTER:	
G)	(2)	(3)	(Ŧ)	(5)	(9)	(2)	(8)	(8)	(00)	(10)	(32)	(1.3)	(47)	(57)	(91)	0.70	(18)	(61)	(50)
ामका सम्बद्धाः	Inch			7.64	Inch	77.32	Inch		: : : In loadin	Party :	: Inch per : : Inch per :	P. 6. 4.		210-3	A 10.00	Production			Dismeters
4/7	0.125	£	Av.	2,065 : 2,590 : 1,491 :	0.00207	3,387 : 4,140 : 2,733 :	0.0196 .0327 .00700.	19.0	16	2,652	0.00361	3,432 : 2,923 :	.55	868 1,019 589	501	8 6 6	0.92	66.0	2,00
1/1	.315	8	Av. Max.	2,163 : 2,600 : 1,646 :	.00380	3,897 : 4,574 : 3,155 :		2004	57	2,374	.00360	3,611 : 4,235 : 2,978 :	S. C. C.	1,023	1493 1493	5.5	.91	1.08	8.8
1/4	.590	15 :	Av. Max.	2,405 : 2,760 : 1,982 :	.00415 : .00525 : .00305 :	4,588 : 5,099 : 3,811 :	.0481 .0615 :0190	*44	Cu pel	2,398 : 2,726 : 2,161	.00491	3,849 3,470	525	1,250 808	532	80 d 34	1.00	1.19	25
1/2	.125	_{ال}	Av. Max.	1,627 : 2,308 : 1,522 :	. 00290 . 00430 . 00430 . 00173	3,017 : 3,504 : 2,380 :	.0123	42%	N	2,756	.00365 .00540 .00183	3,361 4,270 2,792	25 to	1.234	1448 1433 120	2.86	98.	<u>&</u>	1.50
1/2	.315	**************************************	Av. Max.	2,345	.00805 :	3,794 : 4,310 : 3,257 :	0.0925	girik 	a	2,299 r 3,145 : 1,602 :	.00395	3,555	920	1,106	E82	0.4°F	7°-0	1.07	5.00
7/5	. 590	15 :	Av. Max.	2,309: 2,726: 1,924:	.00455	4,639 : 4,170 : 1	. 0656 . 0840	844	ä	2,322	00537	3,852 4,130 3,512	345	1,107	22.2. 2.2. 2.3. 2.3. 2.3. 2.3. 2.3. 2.3	0.4.8	66*	1.8	2,00
3/1	.125 :	52	Av. Max. Min.	2,217 : 2,960 : 1,779 :	.00359	3,479 : 4,008 : 3,018 :	.0133	9	8	2,335 : 1,503 :	.00593	3,647	252	1,467	8.84	8.1 9.0 7.0	• 95	÷.	1,50
7/6	.315	8	Av. Max.	1,927 : 2,205 : 1,793 :	.00334 :	3,261 : 3,550 : 2,930 :	.0301	Big is		2,352: 3,225: 1,809:	.00417 .00645 .00263	1988	龙岩岩	851 1,030 680	99.66	7.5	58.	.95	2.00
3/4	.590	2	Av. Mex.	2,015 : 2,354 : 1,698 :	.00322 .00425 .002425	3,381	. 0560. . 0360.	8.3.3	쿠 	2,514	.00549 .00420	25.75 25.75 25.75 25.75	425.4	1,107 1,107 1,758	15.554	9.6	8.	1.03	2°00
••	••				••	**		Moditied es	Cartesanive :	load that -	End isorgin		Ţ,	į	610) 14	74	,	·	
\$.325	9	Max.	2,519 : 2,805 : 2,160 :	. 64500. . 67600.	4,314 3,580 3,780 1.087	.0820	\$ 50 F	a ====	2,525	00286 00100 82200	3,040	24.8	791 679 667	523	0.8.6	1.12	 	
1/1	965.	9	AV. MAX. Min.	3.550:	. 00147 . 00102 . 00555	5,186	.0940 :	388	6	2,628	35,000°,	3,881 4,260 3,500	82.5	366	525	7.55	1.16	1.34	
1/2	. 28.	61	Av. Hex.	2,300 : 2,065 :	.00479 .00520 .00428	3,387	.0295	2000	5	2,198 2,550 1,910	0.500 0.0051	3,810 :	£69.	882 658	393	7.6	1.05	.97	
1/2	86.	S	Av. Nex.	2,692 1 3,030 2,440	.00663	1,411 : 1,980 : 3,910 :	. 0486 . 0860 . 0280	23.5	9	3,115	.00502 .00502	3,880 : 4,370 : 3,190 ;	\$43	831 1,038 671	200 H 200 H	04.5	1.05	1.1 ¹	
# #	315	g 	Av. Hax.	2.55 ⁴ : 2.975 : 2.280 :	.00594	3,772	.0357 .0450 .0190		g = = = :: :	2,385	.00552 .00600 .00450	3.517	3,8,8	765 909 678	194 124 124	7.5	17.51	1.07	
3/4	.590	A	AV. HAX.	2,494 : 2,815 : 2,260 :	.00656	4,165 4,816 3,700	.0500	86%	9	3,180	.00636	3,830	85.0	1,087	\$ 17.5°	8 8 8 200	. 89	1.01	
1									-										3

pased on weight when even dr. and volume at test.

Table 6 .-- Summary of bolt-bearing stresses

Nominal thick- ness of plywood			At p	roportion	al limit		:		At ultima	te	
	bolt	Yellow birch			ir	poplar	: birch				Yellow-
	: :	<u>1</u> 0°		: 45°	: 90*	0°	: 0°	: 0°	: 45°	: 90°	0.0
	Inch	P. s. i.	P. s. 1.	P.s.1.	P.s.1.	P. s. i.	P.s.i.	P.s.i.	P. 8-1.	P.s.1.	F.s.i.
20.125 .125 .125	: 1/2 :	3,780	2,610		· · ! : · ! : · ! :	2,090 2,250	: 5,860 : 5,650	3,900 3,690			3,320
	: 1/2 :	4,510	2,570	: 2,780 : 2,550 : 2,620	2,830 2,690 2,650	2,310 2,260 2,180	: 9,270 : 7,710 : 7,390	4,330 3,880 3,830	4,600 : 3,870 : 4,020 :	4,580 3,390 3,960	4,040 3,660 3,470
	: 1/2 :	4,110	2,820		::====================================	2,650 2,300	8,960 : 7,630 :	4,710 4,250			4.690

Angle between direction of load and face grain of specimen.

Table 7 .-- Average ratios of bearing and compressive stresses at 0°

iatio:	Diameter:						Specie	s of ply	wood				
į	bolt	Ye	llow bir	ch		Douglas-	fir	: Ye	llow-pop	lar	:Average	for thre	e species
1						Nominal	thickne	se of pl	ywood (i	nch)			********
		0.125	: 0.315	: 0.590	0.155	: 0.315	: 0.590	0.125	: 0.315	: 0.590	: lo.125	: 0.315	: 0.590
A ;	Inch	<u>Ra</u>	: tio F	roportion	mal limi	: t stress	in bear	i ing to p	: roportio	: inal limi	: t stress	in cospr	ession
:	1/4 : 1/2 : 3/4 :	1.03	: : 1.38 : 1.34	1.28	0.88	: 0.94 : .85	0.93	: 0.99	1.02	1.08	: 0.97 : .92	: 1.11 : 1.06 : 1.03	: : 1.10 : 1.00
в :			•	Ratio	Ultimate	: s stress	in bear	in _e to u	: ltimate :	: stress i	: n compres	: sion	:
:	1/4 1/2 3/4	1.13 1.00 .96	: 1.20	: 1.36 :	. 84	. 88	92	1.03 93		1.12	: .92	: 1.22 : 1.04 : 1.00	1.13
c :	:		Ratio	Propo	rtional	limit s	: tress in	bearing	to ultin	: <u>cate str</u>	: ess in co	: mpression	<u>:</u>
:	1/4 1/2 3/4	-64 -64 -58	78 70	: .78 : : .72 :	.66	.63	.69 .61	.62 .59	.68 .65	.70	: .64 : .60	. 70	•72 •64
D :	:		<u>Ra</u>	tio Pr	oportion	al limit	stress	in bear	ng to u	timate	: stress in	: _bearing	
:	1/4 : 1/2 : 3/4 :	.54 .60 .63		: .56 :	.67	.66	.65	.62	.59	. 58	63	58 61 62	.60

¹ Includes values for 0.155-inch Douglas-fir plywood.

²Includes values for 0.155-inch Douglas-fir plywood.

Z M 68633 F

Plywood :	Bolt.	p⊷ı				ouglas-fir		Z Z	1104-	Yellow-:: Yellow :	 8 d	н	Douglas-fir	-fir		. Ye	Yellow-	P-1	. : Yellow :	!		Douglas-fir	fir	1	Yellow-
thicknessidiemeter::	diemeter		lo.	0	: 45°		906	od ::	bren.	0		ô	00 г 450 г		8	S	D to	• ••	0			: 00 : 450 :	8		ô
		-			Ļ.	İ	1	ļ		<u>.</u>	4.00 [-	1]		::::		ļ.,		1			
			•	End o	DRIEID	1	Tensile	co.	**	••	SHER	Clea	Edge clearance	Ĩ	Tenaile	an I		**	핆	FARe cl	SALA	clearance	Compres	cession	al
			••						**	••								**			**		••	**	
20.125	#/1	ณ่	5.	2.00		:		٠. :	8	1.25	**	1.25	*****	-			1.25	**	1.00	ri H	1.25 1		:	****	1.25
	1/2	رن ::		1.50	:		*****	٠ :	8	1.0	8.0	1.00	*****				8	**	3.00	. 7,	8		*****	:::	1.00
	3/4	1.	1.50	5.00		:	*****	.: 1,	1.50	*******	44.044		Sec. 1		***	1	*****	14 P	1.00	7	8			***	1.00
••	ì	**	•••			**		**	**		PH		ė	M		**		**		4+	**		-+	-+	
. 315	4/1	3	2.50	2.50	.,	8		•	8:	: 1.2		1.25		2.00 ::	2,00		1.25	••	1.85	-	1.00 1	8.8		1.50	1.33
	1/2	ผู้	2,00	2.00		8	1.50	••	8	1.25	64	1.00	14	9	1.25	••	8	**	1.00		8	1:33	41	8	9.
. ••	4/4	.: 1.	1.50	5.00	**	1.50 :	1.50	••	1.50				*****		****	+	1 4 + + +	**	1.8	į.	8	1.50	ä	8	1.00
• • •	ì	**	•••		**	**			**	**	**					**		**		**	44		**	94	
. 590	1/1	2	2,50	2.50	:		*****	<u>ر</u>	8	: 1.5	h IP	1.25		:		1 ::	1.25	**	1.3	i.	1,00 1	:		:	1.25
	1/2	3	2.00	8.8	:	::		<u>ري</u>	8.00	1.00	10	90:	:	:		ਜ ::	8	**	1.25	7	8			::	1.8
••	3/1		1.75	2.8		:	******	ď	8	2+++4							:		3.00	i.	8				1.00
		44						*1	**		11		**					41		**	*				

Angle between direction of load and face grain of specimens. Encludes 0.155-inch Douglas-fir plywood.

Table 9. -- Summary of bolt-bearing deformations

Nominal Mismeter:	il)ismets			4	At proportional limit	lone	1 limit					7	At ultimate	timet	0	
of	bolt:		Yellow		Douglas-fir	8-£1	i i	, M	Yellow-		Yellow	Ä	Douglas-fir	8-f11		Yellow
pramood	** **	** **	10 e CE	!	0 : μ5		906	Ď,	poptar 0		0110	0	. 25°		06	poptar.
Inch	Inch		Inch	[□ .	Inch Inch	,ai	Inch		Inch	: :: :	Inch	Inch	पुरुषा		Inch	Inch
20.125	1/1				0036:	-		_	,0023		0.0189	:0.0119		. :		
	2/7	**	2400·		0027 ;	1			.0026	**	.0165	.0102:***********************************	+ + + + + + + + + + + + + + + + + + + +			.0123
	7,4	no (500.		00.5% 00.5%				200		10.0	1410.	*	:	***	
	¥4.	•• •	100	•		:			.0067							
.315	1/4		.0045		: 300.0: 0500.0:3400. : 5400.	5	0.00 JE		.0042		1610。	: .0164:0.0253	£0.02	53 :0	0.0301	: .0423
	1/2	**	.0039	•	0039: .00	ぎぶ	5+00°	94	1400.	**	.0311	: .0126	: .02	33	6910. :	: .0422
	3/4	••	100.	•	oo. :61,00	ß	.0052		††00°	**	.0251	: .0143	. 02	34 :	.0154	0320
	. Av.	••	.0043	**	.0045: .0C	52	4100	(22	· 0042	**	.0352	: .01\#	: .02	 였	.0208	: .0388
		••		44						**		••		14		••
.530	7/1		2900	**			*****	-	.0058	**	.0728	: .0370:	;			: .0495
	1/2	**	1100	**	.0043i	:	*******	10	·0043	**	.0624	: .0188		-	*****	0539
	3/4	**	.00h8	**	.0048:		******		.0051	**	.0587	: .0162				+ -0554
	Av.	**	. 0053	••	.0052	1	********	**	.0051	**	.06½	: 0540:	1	-	:	0529
																-

Angle between direction of load and face grain of specimen.

Table 10.--Basic stresses and comparison of loads computed by the formula P=(T-t)(DS+2C)
with the loads obtained by test

			-						
Type	: Average :		Proport	ional li	mit		נט	timate	
test	: of :	Load ²	Basic :	Computed load—	: Deviation : of computed : load from : Col. 3	Load ²	Basic 3	Computed: load—	Deviation of computed load from Col. 7
(1)	(2)	(3)	(4)		: (6)	(7)	(8)	(9)	(10)
	Inch				: Percent	Pounds			-
	9 :		<u>G</u>	OUP I PL	YWOOD (YELLO	BIRCH)	in i		ě.
			No	minal bo	lt diameter=	1/4 1ach			
TC C TM TC C TM MC TC C TM	: .131 : .274 : .291 : .290 : .272 : .602 : .606	125 104 321 327 338 338 617 666	4,067 : 3,897 :	125 125 301 322 321 299 706 711 707 701	+ 5 + 20 + 20 - 6 - 2 - 12 + 14 + 4 + 4 + 10	199 239 641 573 602 664 1,338 1,320 1,384 1,533	7,128 7,724 6,639 6,401 7,366 6,338 6,270 6,648	230 230 585 580 625 627 1,398 1,388	- 4 - 9 + 1 + 6 + 4 + 5 + 1
			NO	mipal bo	1t dismeter=	I/Z INCH			
TC C TM TC C TM MC TC C TM MC	: .287 : .279 : .276 : .602 : .603 : .602	255 220 220 266 264 2642 2654 21,255 21,157 21,249		225 232 575 596 577 570 1,326 1,328 1,328	: - 7 : -10 : -13 : + 6 : +15 : + 6 : - 1	372 397 1,016 944 1,028 990 2,343 2,296 2,378 2,596	6,359 6,961 6,375 6,736 6,612 6,861 7,624	384 371 1.001 993 1.005 1.039 2.353 2.340 2.353 2.353	: -1 : +5 : -2 : +5 : +5
					<u>0 = 0°</u>				
C TM C C TM MC C TM MC	: .600	: 331 : 847 : 905 : 969 : 2.013 : 1,632	4,665 5,095 4,495 3,615 3,965	337 854 844 830 1,946 1,940	: +2 : +1 : -7 : -14 : -3 : +19	: 1,373 : 1,455 : 1,421 : 3,033	6,810 : 6,710 : 7,037 : 6,747 : 6,144 : 6,590 :	539 1,384 1,407 1,425 3,296 3,296 3,307	

est ¹	of :	T2							
-		Toad= :	Basic ;	Computed:	Deviation :	Load ^E :	Basic :	Computed	: Deviation
:	specimens:	:	stress-	load <u> :</u>	of computed:	: :	stress-4:	load-	: load from
					Col. 3		:		: Deviation :of computed : load from : Col. ?
(1)	: (2) :	(3)	(4)	(5)	(6)	(7)	(8)	(9)	: (10)
	: Inch :	Pound s	P. s. i. :	Pounds :	Percent	Pounds	P. s. 1. :	Pound s	Percent
i	•	•	G)	minal bo	YWOOD (DOUGI t diameter=) $\theta = 0^{\circ}$	AS-FIR) /4 inch	•		•
rc :	0.151		2,199	108		161	3,714	161	
C:	: .154 : : .153 :		2,792 2,127				4,508 : 3,643 :		
rc :	: .318 :	220 :	2,219 :	235	4.7	334	3.377	359	: +7
C:	: .322	248	2,547	238	- 4	339	3,384	363 365	
	: .323 : : .316 :	244 :	2.557	234	0 - 4 +15	160	3,759	356	
C	605	384	1,940	454	+18	717	3,835	356	: - 3
C M	: .60 ¹ 4	492	2.687	54	+ 8		3,690 :	697	
EC .	596	486	: 2,687		- B	673	3,604 :		
	:		:		θ = 45°		:	i.	:
	: .320	228				388	,		;,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
C	: .317 : .321	234:				372			1
	:			:	<u>θ = 90°</u>		:		:
PC D1	320	211			.,,,,,,,,,,,	372			: :
С	: .316	226				347			;
4C	: .319	235	:	:) 1 ⁴			: -
			_		A = 0°				
			: 1.975	193	+19	269	3,443	287	
C PM		: 184 : 209	2.238	189	-10	301	3,690 4,013 3,742	393 281	- 7
rc	: .320	145,1	: 2,421	: 424	. 0	649	3.742	644	: - 1
			: 2,501	1 423	= 3	: 618 . 62h	3,550	641 648	The state of the s
	314	405	: 2,351	416	+ 3	651	3.842	631	- 3
	: .604	: 824	: 2,456	: 813	- 1	1.239	3,694	1,243	: 0
	: .603	786	: 2.311	819	+ 4	: 1.247	3.692	1,251	. 0
MC	590	876	2,706	794	- 9 - 3 - 1 - 4 - 4 - 9	1,210	: 3,695	1,213	; 0
	:		:	•	θ = 45°	•	•	•	•
		358	*******			563			
(C	: .319	: 393			:	658			
	:	:	· Fi	:	: <u>0 = 90°</u>	:	:	•	1 ♥
TC	. 321	397		9 	i 		: 9 Sanaherei		8 1
		402		1			2		
MC	: .318	: 462		:	:	5	:	:	:
			N	ominal bo	$\frac{\theta = 0^{\circ}}{\theta}$	3/4 inch			It s
С	3.63	. 268	: 2,325	· 278	4 4	409	3,669	413	+ 1
TM	: .149	: 266	: 2,343	: 2/4	: + 5	: 419 : 898	: 3.825	: 407 : 923	+ 3
C TM	: .319		: 2,330		+ 4	: 887	3,546		: +4
MC	312	: 663	: 2,718	: 596	: -10	. 962	· 3.975	: 902 : 1.795	: -6
C TM	: .606	: 1,086	: 2,215	: 1,177	: +6	: 1.846	: 3.779	: 1,813	: - 5
MC	: .589	: 1,385	: 2,982	1.143	z -17	: 1,817	: 3.878	1.743	F = 18
	:	:	:	:	<u>θ = 45°</u>			:	
С	: .318	: 545	1		4 4				
MC	319	: 696		Acres as a		; 985	4	:	
	:	:			θ = 90°	:			
	: .314	: 570	:	:	: :	898	:		: .2
С									

Table 10.--Basic stresses and comparison of loads computed by the formula P=(T-t)(DS+2C) with the loads obtained by test (Continued)

Туре	Average :		Proporti	onal lim	it		1	Ultimate	
test-	of specimens	: Load≤	: Basic :: stress3:	Computed: load	Deviation of computed load from Col. 3	: Load ²	stress4	: load-	of compute load from Col. 7
(1)	(2)	: (3) :	: (4) :	(5)	(6)	: (7)	: (g)	: (9)	(10)
;	: Inch :	Pound s	: P.s.i.	Pounda	· Parcent	Pounda	Pot.	· Pounda	Pomoont
		i .	GRO	UP III P	: LYWOOD (YELLA	: DW-POPLAI	: R)	1	
					lt diameter=		24,		
					0 = 0°	9			
TC-	0.128	7 <u>1</u> 1	. 2 272	79	- 3 - 5 + 6 + 5 - 1 - 1 - 4 + 1 - 7	116	7 701	117	4.1
C	.129	77	: 2.360 :	77		125	. H 016	. 11/ 7	7 5
TM :	.129	69	: 2.074 :	73	+ 6	114	3.589	: 118	4 11
TC :	. 320	185	: 2,076 :	194	+ 5	367	4.212	334	- 9
C :	. 316	194	: 2,231 :	192	- í	318	3.599	: 329	+ 3
TM :	.315	166	: 1,859 :	191	+15	303	3,403	328	+ 8
MC :	. 320	207	: 2,370 :	194	: -6	359	4,102	: 334 :	- 7
TC :	598 :	388	: 2,316 :	371	: - 4	686	4,029	: 647 :	- 6
C:	604	370	: 2,163 :	375	: + <u>1</u> ;	: 607 :	3,425	: 654 :	+ 8
MC :	604	: טפנ	: 2,025 ;	210	+ /	040	3,050	654	+ 2
:	.002	102	: -1))7 :	דוכ	- 1	170	7,220	, 45T	- 9
					lt diameter=			'	
					$\theta = 0$				
TC :	.129	1)10	: : 2,283 :			000			
TC :	127	110	: 2,20):	174	- + 3	220 3	3,812	21/ :	- 1 0
TM :	.128	128	2.092	1 2/1	15	207	7,170 i	217	+ 4
TC	.315	330	: 2.024 :	356	+ 8	616	3.858	601	- 2
C :	.320	381	: 2.324 :	362	- 5	620	3.812	611	- 1
TM :	.315 :	381 :	: 2,366 :	356	- 7	605	3,782	601	- 1
MC :	.312 ;	380 :	: 2,386 :	352	- 7	543 :	3.391	595 :	+10
TC :	.603 :	620 :	: 1,932 :	698 :	+12	1,255 :	3,958	: 1,196 :	- 5
C :	. 603 :	574 :	: 1,774 :	690	+22	1,049 :	3,241	1,196 :	+14
TM : MC :	.601 :	696 :	2,200 :	695 :	: 0	1,294 :	4,110	1,192 :	- g
MC :	• 590 :	132:	: 2,341:	689	- 4 - 4 + 5 + 8 - 5 - 7 - 7 + 12 + 22 - 6	1,219 :	3,885	1,181 :	- 3
•	•				t diameter=			:	
					0 = 0°	22 :			
. :			2 75.50	224.692	: :			:	
C:	.129 :	219 :	2,441 :	198	-10 :	324 :	3,864 :	315 :	- 3
_	.130 :	216 :	2,388 :	200	- 7	333 :	3.937 :	318 :	- 5
C: TM:	719	Du7 .	1 888	516	→ 0 :	771	5, (52 :	891 :	0 -11
A IVI) 0	.315	670	2 866	521	-10 :	900	י עונס ד	877 ·	-11 - 4
MC .	- 7-7 4	ALA .	-1007 1	Jer .	-22 ;	207 1	71747 8	- 213 .	
MC :	.603 :	952 :	2.042 :	1.021	+ 7 •	1.627	3,494 :	1.736 ·	+ 7
	.603 :	952 : 834 :	2,042 :	1,021 :	-10 : -7 : -6 : -15 : -22 : +7 : +20 : +8	1,627 :	3,494 : 3,609 :	1,736 :	+ 7 + 4

Designation of type of test.

Sheet 3 of 3

TC = tensile loading - edge clearance.

C = compressive loading - edge clearance.
TM = tensile loading - end margin.

MC = modified compressive loading - end margin.

Zhoad values adjusted to average strength of control specimens.

Astress computed on effective thickness after subtracting edge support from load.

 $[\]frac{14}{2}$ Load computed by formula P=(T-t)(DS+2C) using average value of basic stress.

^{5 =} angle between grain of face plies and direction of applied load.

Z M 68637 F

Table 11 .-- Values of the basic bolt-bearing stress (S), the edge support (C), and the effective thickness (T) and of the relation of the basic bearing stress to the compressive stress

		••										•••	
Species	Grain	44		Basi	c beart	Basic bearing stress	00 07			Average compressive		Basic bearing stress	stress.
	ergue :		1 4				Ė	TO timote	, , ,		100 T	: compressive stress	BELGER
		rodora .	277074	DITT 42	•		1	0000000		Danamttonal.	171 + 4 2 2 4 5	· Decompany one 1 11 + function out to contract of the function of the functin of the function of the function of the function of the function	TI + 4 me to
	P# 40 (0			E-1	တ			E⊀	limit		: limit	540000000000000000000000000000000000000
	Dekraes	Degrees: P.s.i. : Pounds : Inch : P.s.i. : Pounds : Inch : p.s.i. : por inch : i.per inch :	Por	Pounds Der inch	Inch	0.1		Pounds per inch	Inch	•			
Tellow birch	•	: 4,336		 	0.030: 6,770	6,770		204	: 0.038:	3,453	5,945	1.26	1.14
Donel as-fir	0	2.422	•• ••	 	.010	902.3.706	•• ••	136	.015		17715-17	.75	60
		*	• • •	••			•••	132 :		2,982	4,358	62.	58.
	. 190	•	••	31 :			ch	132		2,963	7,260	83	. 86
: :Tellog_mofler	•	2.199			.015	.015: 3.752	•• ••	102	105h	5. 249	7.617	ক •	1.04
1										- 1			

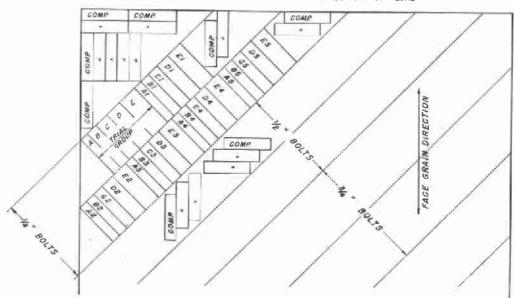
Values based on assumption that "I" for 45° and 90° is the same as for 0°.

Z M 68638 F

(A) FACE GRAIN O' OR 90" TO DIRECTION OF LOAD

GROUP	51708	T awos		8	E	COMP	D	Ε	COMIR	10	18	Ġ	COARP	10	13	COMP	42	88	02	COMP	02	Ci lii	CUMP	m m	2 0	COMP	50	63	GOATE
- GP	9 . %	2,800	P 4	B&	P ()	COMP	0.0	4.0	60479	A	8.5	63	SOME	90	4.5	COMP													
	- STJ08 " 4-+											_		_												FACE GRAIN DIRECTION			
	% " BOLTS								_											-						FACE GA		_	

(B) FACE GRAIN 45° TO DIRECTION OF LOAD



L SPECIMEN FOR COMPRESSION TEST

Figure 1.--Cutting diagram showing manner of cutting trial groups, final groups, and series for the three grain angles used.

2 M 69369 F

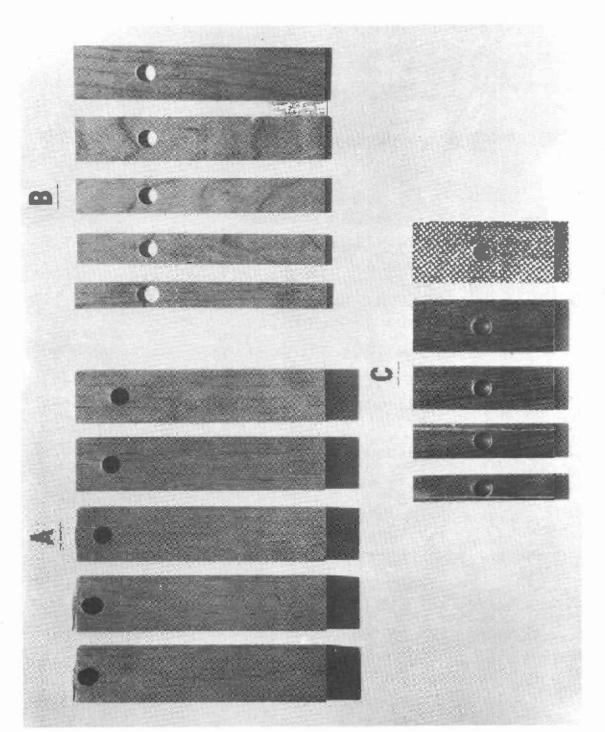


Figure 2.--Specimens after test showing the five variations of end margin or edge clearance used in three different groups: (A) end margin determination by tensile loading; (B) edge clearance determination by tensile loading; and (C) edge clearance determination by compressive loading.

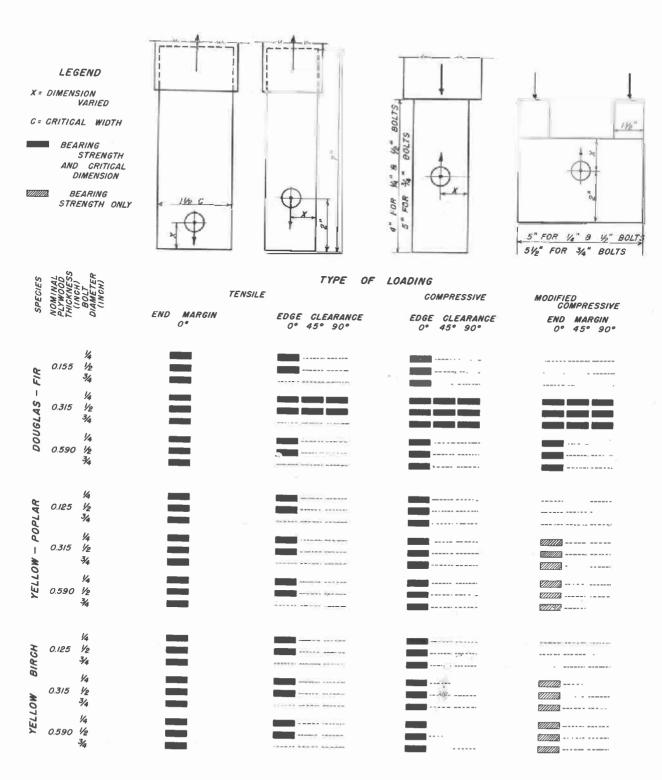


Figure 3.--Diagram of the four types of specimens showing manner of load application and the extent of the testing with each.

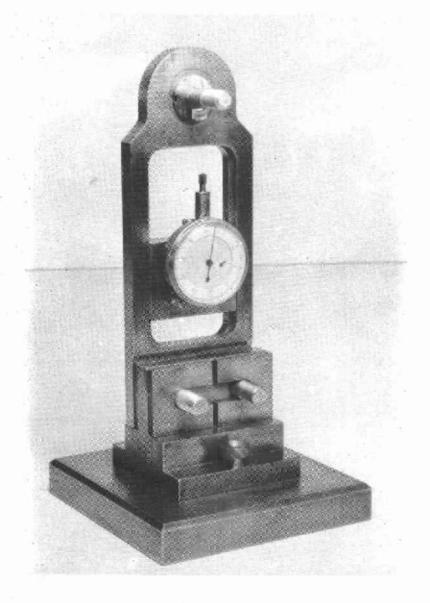


Figure 4.--Jig for bolt-bearing tests with test bolt in place but one plate and bushing removed. Note spacers, dial support and bushing with projecting lug.

Z N 69874 F

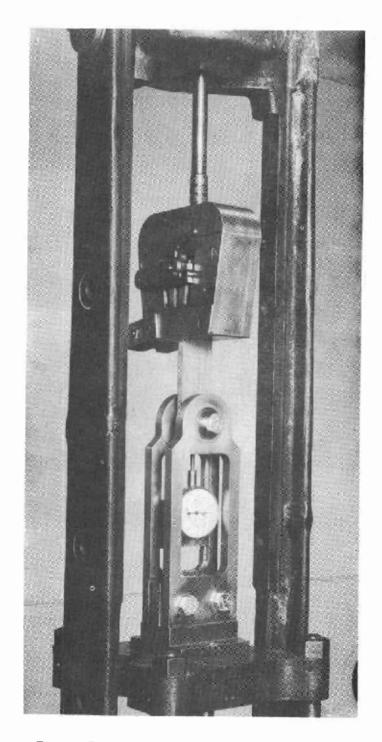


Figure 5.--Setup for tensile loading of bolt-bearing specimens. Note spacing between plates and specimen and also between bolt head and bushing.

Z M 69875 F

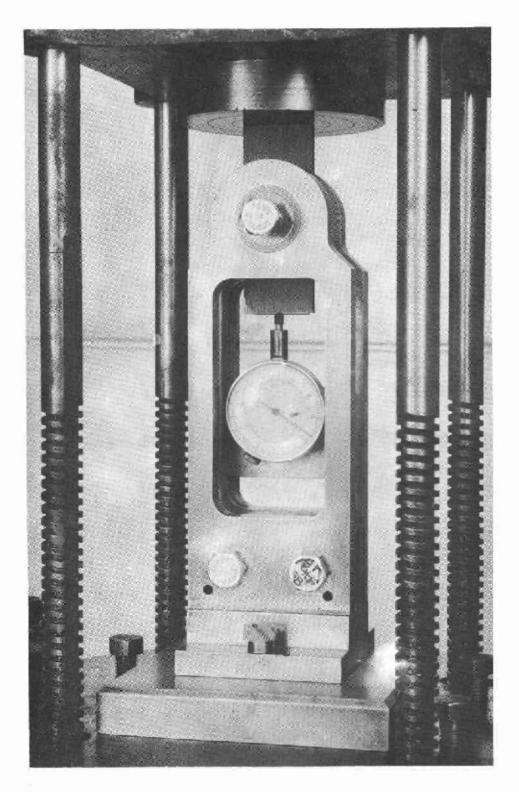


Figure 6.--Setup for compressive loading of bolt-bearing specimens.

Z M 69876 F

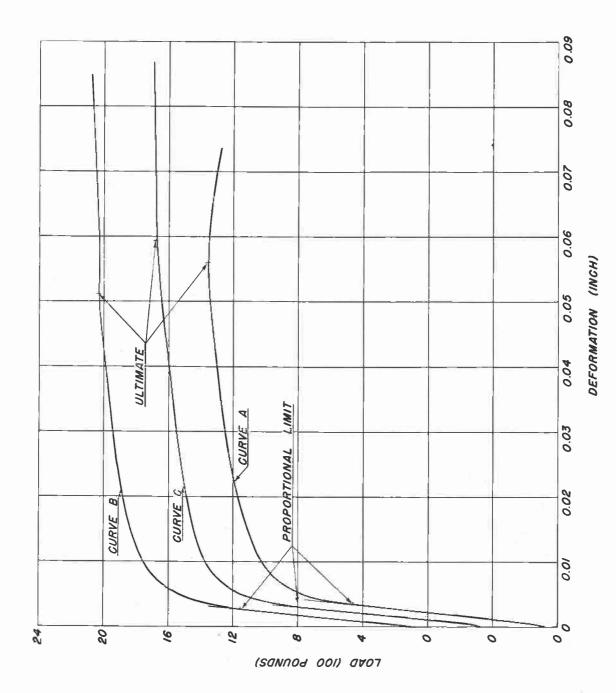


Figure 7.--Typical load deformation curves of three types showing selection of ultimate load for each.

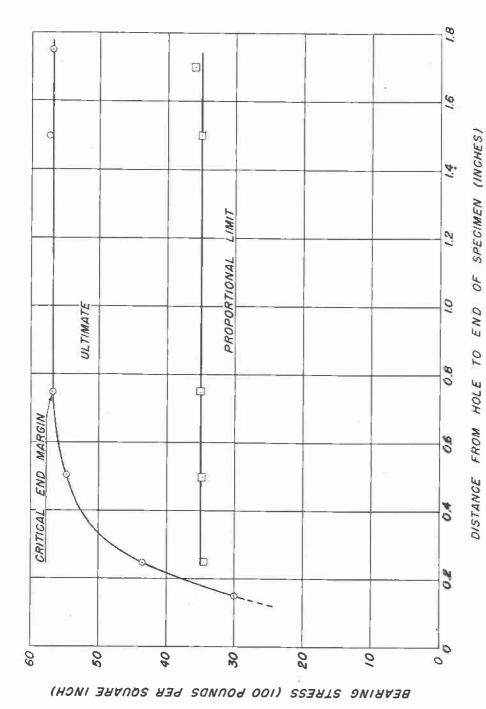


Figure 8.--Typical relationship between the distance from the hole to the end of the specimen and the bearing stress at the proportional ilmit and ultimate.

1 24569 M Z

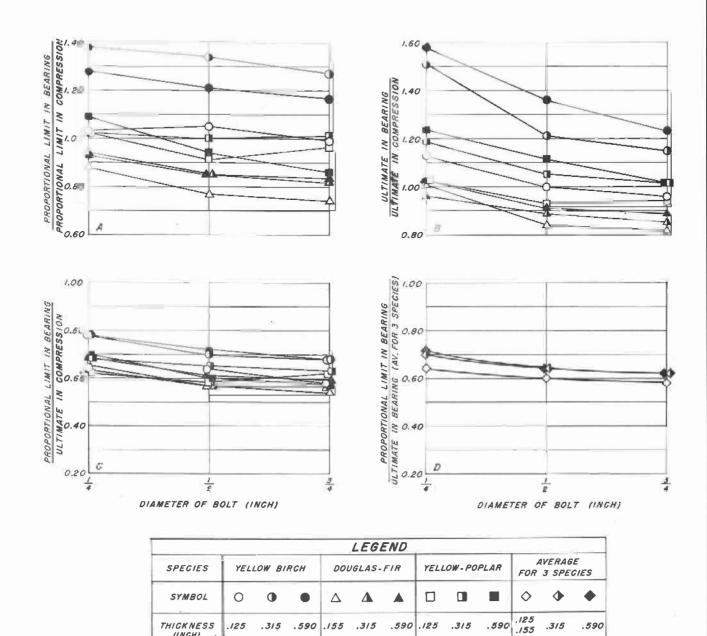


Figure 9.--Relation of bearing stress to diameter of bolt for the three species and thicknesses of plywood.

(INCH)

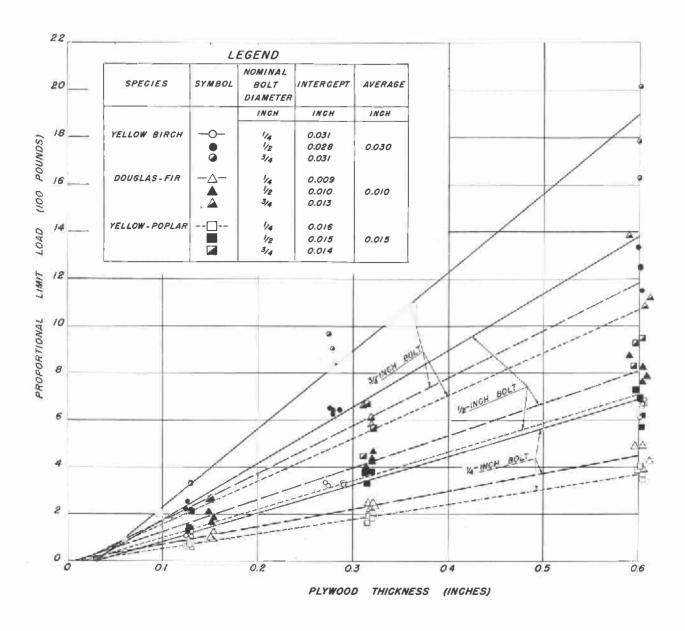


Figure 10.--Relationship between proportional limit load and thickness of plywood for each species and bolt diameter.

Z M 69374 F

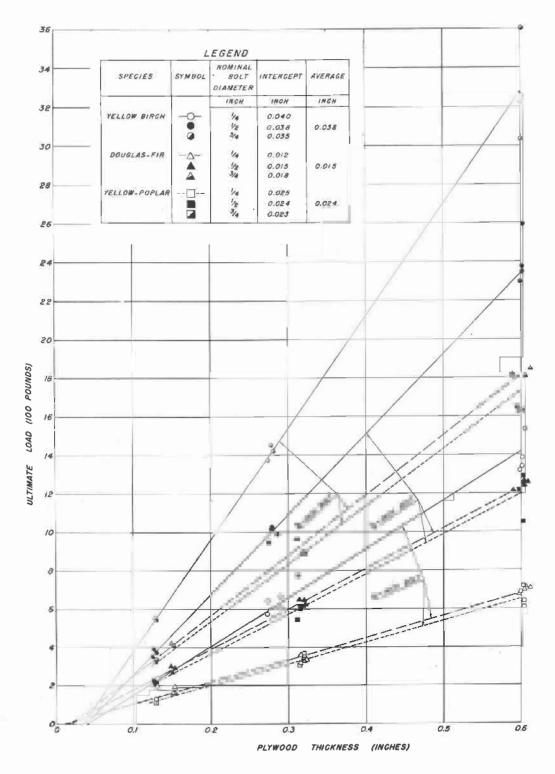


Figure 11.--Relationship between ultimate load and thickness of plywood for each species and bolt diameter.

Z M 69375 F

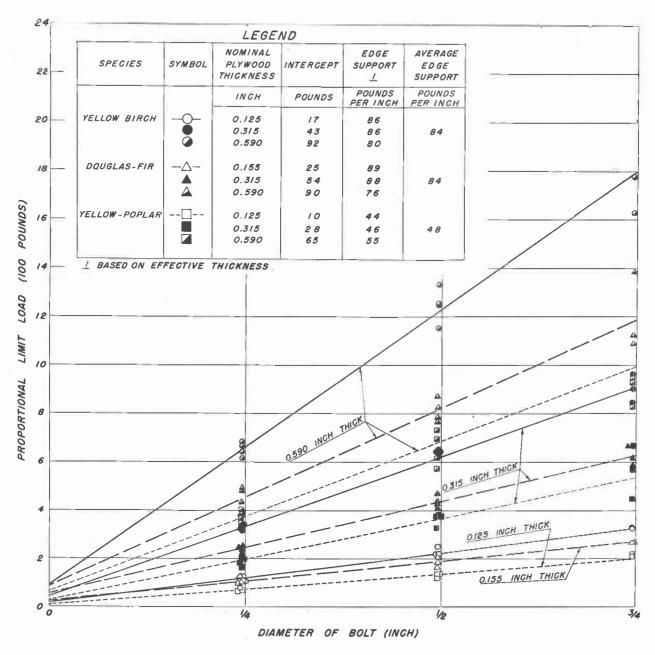


Figure 12.--Relationship between proportional limit load and diameter of bolt for each species and plywood thickness.

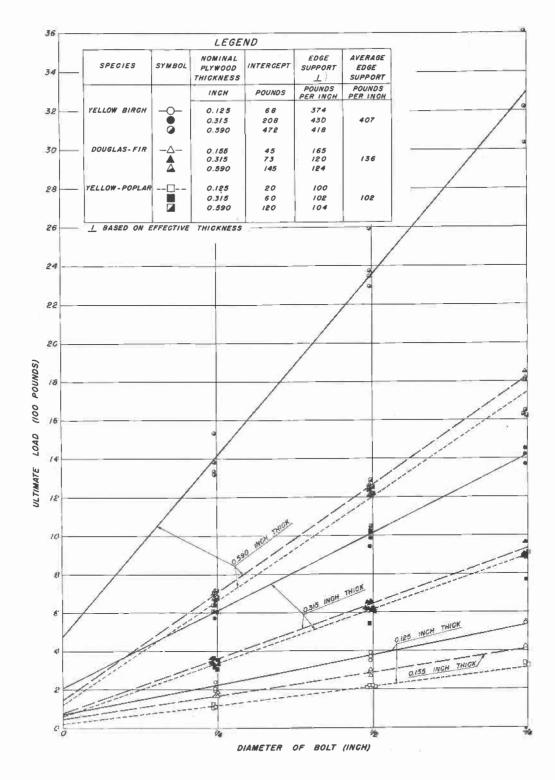
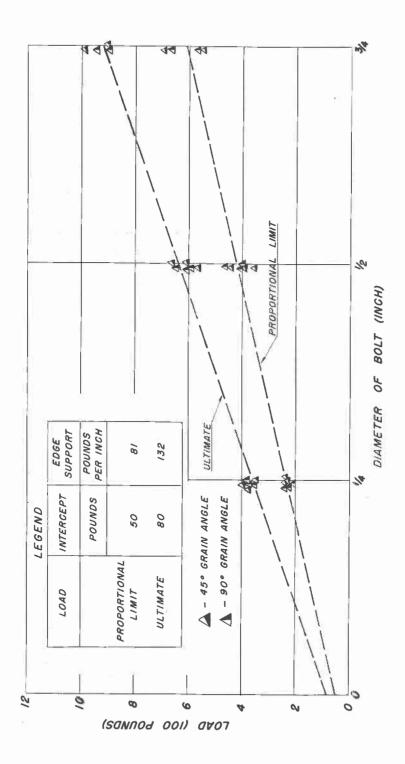


Figure 13.--Relationship between ultimate load and diameter of bolt for each species and plywood thickness.

z M 69377 F



proportional limit is 50 pounds and at ultimate is 80 pounds per inch. Edge support at proportional limit is 81 pounds and at ultimate is 132 pounds per inch. the diameter of the bolt for 0.315-inch thick Douglas-fir plywood when the angle of the face grain is 45° and 90° to the direction of loading. Intercept at Figure 14. -- Relation of the Loads at the proportional limit and at the ultimate to

2 M 69378 P