BOLT-BEARING STRENGTH OF WOOD AND MODIFIED WOOD

BEARING STRENGTH OF COMMERCIAL

CROSS-BANDED COMPREG UNDER

AIRCRAFT BOLTS

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In Cooperation with the University of Wisconsin

BOLT-BEARING STRENGTH OF WOOD AND MODIFIED WOOD

Bearing Strength of Commercial Cross-Banded

Compreg Under Aircraft Bolts 1, 2

By

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Summary

This report presents the results of approximately 1,800 bearing tests on single-bolt specimens of commercial cross-banded compreg under steel air-craft bolts.

In this investigation, the bearing strength and the critical dimensions (minimum edge clearances and end margins required to develop the full bearing strength) were determined for compregs from three manufacturers (desig-

- ¹This is one of a series of progress reports prepared by the Forest Products Laboratory relating to the use of wood in aircraft. Results here reported are preliminary and may be revised as additional data become available. Report originally published February 1946.
- ²This is one of a series of reports dealing with bolt-bearing strength of wood and modified wood. Other reports are No. 1523, "Effects of Different Methods of Drilling Bolt Holes in Wood and Plywood;" and No. 1523-A, "Bolt-Bearing Strength of Laboratory-Made Cross-Banded Yellow Birch Compreg Under Aircraft Bolts." Other reports will be issued as data becomes available.

³Maintained at Madison, Wis., in cooperation with the University of Wisconsin.

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nated in this report as A, B, and C), three grain orientations $(0^{\circ}, 45^{\circ}, \text{ and } 90^{\circ})$, three thicknesses of compreg (1/4-, 1/2-, and 1-inch), and three diameters of bolt (1/4-, 1/2-, and 1-inch), under one or more of three methods of loading: compressive, modified compressive, and tensile.

Edge clearance, as referred to in this report, is the dimension, perpendicular to the direction of loading, from the center of the bolt hole to the edge of the member. End margin is the dimension, parallel to the direction of loading, from the center of the bolt hole to the free end of the member.

The average bolt-bearing strength for the three commercial compregs investigated was only slightly less than that for laboratory-made compreg presented in Report No. 1523-A; the critical dimensions were about the same; the compressive strength parallel to the face grain was about 20 percent less than that for laboratory-made compreg as presented in Report No. 1523-A and about 25 percent less than that shown in table 2-14 of ANC Bulletin 18. The commercial compregs appeared to be more sensitive to notching than the laboratory-made compreg.

The bearing strength increased with increase in thickness of compreg, decreased with increase in diameter of bolt, but was practically the same for grain orientations of 0°, 45°, and 90°. In general, the critical dimensions were smaller for 45° grain orientations than for 0° and 90°; they were essentially the same for all thicknesses of compreg, and were approximately in proportion to the diameter of the bolt. Consistent relationships were developed between bearing stresses at proportional limit and ultimate and between ultimate stresses in bearing and in compression. Suggestions are made for determining design bearing stresses for any grain orientation by applying appropriate factors to the compressive strength of cross-banded compreg parallel to the face grain.

Repeated-loading tests employing 1/2-inch bolts in 1/2-inch compreg indicated that bearing stresses approximately one-third greater than proportional limit values can be applied repeatedly with no significant increase in the permanent deformation, or "set."

On the basis of these tests, cross-banded compreg can be expected to develop its proportional limit stress in bearing under steel aircraft bolts at deformations not exceeding 0.025 inch and its ultimate at deformations not exceeding 0.10 inch.

Introduction

The investigation of the bearing strength of commercial compreg, on which this report is based, was undertaken at the request of the ANC Technical Subcommittee on Wood Aircraft Structures to provide design data applicable to compregs available commercially to wood aircraft manufacturers. The information desired included bearing stresses at proportional limit and ultimate under both tensile and compressive loading and the minimum edge clearances and end margins necessary to develop the full bolt-bearing strength of the material. (Edge clearance is the dimension, perpendicular to the direction of loading, from the center of the bolt hole to the edge of the member. End margin is the dimension parallel to the direction of loading, from the center of the bolt hole to the free end of the member.) In addition, it was desired to compare the bolt-bearing characteristics of certain commercial compregs with those previously determined for compreg made at the Forest Products Laboratory.

Description of Material

The commercial compregs A, B, and C used in this investigation were made during the first quarter of 1943 by three reputable manufacturers. All material was cross-banded.

Compreg A was made from yellow birch veneers impregnated with 27 to 30 percent of water-soluble phenol-formaldehyde resin. The number of plies and the thickness of veneer varied with the finished thickness of the compreg, the 1/4-, 1/2-, and 1-inch material containing seven 1/16-, thirteen 1/16-, and thirteen 1/8-inch veneers, respectively. The assembled impregnated veneers were pressed at 1,300 to 1,500 pounds per square inch for 45 minutes. Additional resin was used to improve the bond between plies. The temperature during pressing varied from 275° to 290° F.

Compress B and C were made from hard maple veneers impregnated with approximately 30 percent of alcohol-soluble resin. Information is not available as to whether additional adhesive was used between the plies. Compreg B was composed of 1/10-inch veneers, the 1/4-, 1/2-, and 1-inch panels containing 5, 9, and 17 plies, respectively. The 1/4-inch panels of compreg C were composed of seven 1/16-inch veneers; the 1/2- and 1-inch panels of seven and fifteen 1/8-inch veneers, respectively. Pressures between 2,000 and 3,000 pounds per square inch were used in making compress B and C. The temperature during pressure was approximately 300° F.

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Other detailed information concerning the individual panels of commercial compreg is presented in table 1.

The bolts used were steel aircraft bolts conforming to Army-Navy Specification AN-B-3a.

Preparation of Specimens

The scheme employed in preparing the bolt-bearing specimens was designed to isolate the effects of source of material, thickness and grain orientation of compreg, diameter of bolt, and method of loading on the bearing strength and on the critical dimensions. Specimens were usually prepared in an initial series of three to five in which either edge clearance or end margin was varied over a range intended to bracket the critical dimension. After the initial series was prepared, extra material was set aside so that all additional specimens relating to that particular series could be cut from the same panel, adjacent to the original specimens. If the critical dimension was not determined from tests of such an initial series, supplemental specimens were prepared to extend the range of dimensions until the critical dimension was bracketed. When the critical dimension was thus tentatively determined, one to six additional specimens were prepared to correspond to each of five specimens centered around the critical dimension. The additional specimens, together with those in the initial series, constituted a complete series of from two to seven specimens for each of five different edge clearances or end margins for each combination of diameter of bolt and thickness of compreg. Specimens for compression tests and specific gravity determinations were taken at random from each panel and were presumed to represent the entire panel.

Each bolt-bearing specimen was marked in the manner explained in Report No. 1523-A, with the addition of two qualifying letters. One letter was appended to the first numeral to indicate the source of material, while the other was appended to the final numeral to indicate the order of the specimen in a group for any one condition of test. Specimens were cut, holes were drilled, and completed specimens were conditioned in the manner described in Report No. 1523-A.

General Discussion

The test procedure was the same as that described in Report No. $1523-A^2$ with but few exceptions. In laboratory-made compreg, only one specimen was tested for each condition of test; in the commercial compregs, approximately three specimens were tested for each condition. In the following discussion, only the procedure which differs from that used for the laboratory-made compreg will be explained.

Bolt-Bearing Under Compressive Loading

In compreg A, a complete edge-clearance series was made for each of three grain orientations (0°, 45°, and 90°), and for each of seven combinations of compreg thickness and bolt diameters, consisting of 1/4-inch bolt in 1/4and 1/2-inch compreg, 1/2-inch bolt in 1/4-, 1/2-, and 1-inch compreg and 1-inch bolt in 1/2- and 1-inch compreg. In compreg C, complete series were made for the same three grain orientations but for only five combinations of compreg thickness and bolt diameter. These consisted of 1/4- and 1/2-inch bolts in 1/4- and 1/2-inch compress and 1-inch bolt in 1/2-inch compreg. Additional tests were made employing 1/2- and 1-inch bolts in 1-inch compreg C, with face grain parallel to the direction of loading, to determine bearing strength only. Specimens for these tests were purposely overdesigned with respect to critical dimensions in order to restrict failure to bearing. In compreg B, tests to determine bearing strength only were made for the seven combinations of compreg thickness and bolt diameter employed for compreg A, but only in material having face grain parallel to the direction of loading.

Bolt-Bearing Under Modified Compressive Loading

Only compreg A was tested under modified compressive loading. The combinations tested were those for which tensile loading was not practicable with available apparatus and included 1/2-inch bolt in 1-inch compreg and 1-inch bolt in 1/2- and 1-inch compreg. The three combinations were tested with grain orientations of 0°, 45°, and 90°.

Bolt-Bearing Under Tensile Loading

The specimen was suspended on an aircraft bolt between two fittings of an improved tension jig, and a self-aligning tension grip was attached to the

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lower end (fig. 1). The deformation of the specimen at the bolt hole was measured by means of a single 1/10,000-inch dial gage held directly above the top of the specimen by a cross-bar.

In compress A and C, both critical edge clearances and critical end margins were determined by tensile loading for 1/4- and 1/2-inch bolts in 1/4- and 1/2-inch material and for three grain angles. No tests of compreg B were made under tensile loading.

Tests for Compressive Strength

Compressive tests were made with the grain of the face plies oriented to correspond with that of the related bolt-bearing specimens with the exception of those at 45°, which were made at both 0° and 90° and the averages of the values for the two orientations were assumed to apply at 45°.

Repeated-Loading Tests

Because of apparent differences between tensile and compressive loadings, and between laboratory-made and commercial compregs, with respect to the ratio of proportional limit to ultimate bearing stress, a few tests were made using 1/2-inch bolts in 1/2-inch compreg C under repeated tensile and compressive loadings.

The proportional limit stress was first determined in the usual manner by noting the point at which the load deformation curve departed from a straight line. Matched specimens were then loaded to this proportional limit stress, unloaded, and then reloaded successively to progressively higher loads until stresses exceeding two-thirds of the ultimate were attained. The tests were then carried to failure. A new proportional limit stress was obtained from the repeated-load curves. Additional matched specimens were then loaded to this new proportional limit stress 25 times to determine the extent of increase in the permanent deformation, or "set."

Explanation of Tables and Figures

Information pertaining to dimensions, specific gravity, and compressive strength of each compreg panel from which specimens were prepared is presented in table 1. Column 5 shows the wide variation in thickness of panels 10C and 11C as compared to other commercial panels. A summary of the information obtained from both the bolt-bearing and compression tests is given in tables 2 through 14. Tables 2 through 7 relate to compreg A, table 8 to compreg B, and tables 9 through 14 to compreg C. The data for bolt-bearing tests under compressive and modified compressive loadings are presented in tables 2, 3, 4, 8, 9, 10, and 11 and the data for bolt-bearing tests under tensile loading in tables 5, 6, 7, 12, 13, and 14. Tables 2 through 14 are similar to tables 2 through 5 for laboratory-made compreg in Report No. 1523-A except that the compression test data relating to bolt-bearing tests at 45° in commercial compreg were determined by averaging the values for 0° and 90°.

In table 15 the critical dimensions for commercial compreg are summarized and compared with the critical dimensions for laboratory-made compreg. Tables 16, 17, 18, and 19 are similar summaries of bearing stresses, deformations, ratios of bolt-bearing stresses at proportional limit to ultimate, and ratios of bolt-bearing stresses to compressive stresses.

Figure 1 shows the improved tensile-loading apparatus employing a single 1/10,000-inch (0.0001-inch) dial gage, mounted directly above the specimen, for measuring deformation of the specimen at the bolt hole. Elongation in the compreg alongside the bolt hole and other minor deformations in the specimen and the apparatus are included in the deformation registered by the gage. These contributions are assumed to be comparatively insignificant, and the readings of the gage are accepted as measurements of the deformation at the contact between bolt and compreg.

Figure 2 illustrates the method of determining the critical edge clearance or end margin and the average proportional limit and ultimate bearing stresses.

Figures 3 through 6 show failures for four typical series of edge-clearance and end-margin tests under compressive, modified compressive, and tensile loadings, together with bearing strength data pertaining to each specimen.

Figure 7 is a graphical summary of bearing stresses at proportional limit and ultimate for all compreg studies (FPL, A, B, and C). This chart permits direct comparison of the bearing stresses for the various materials, grain orientations, types of loading, thicknesses of compreg, and diameters of bolts.

Figure 8 demonstrates the relationship of bearing stress to compressive stress for the three commercial compregs studied in this investigation.

Figure 9 consists of two logarithmic charts showing the effects of thickness of compreg and diameter of bolt on the ratio of bearing stress to compressive stress at ultimate. In figure 10A, the points and lines in figure 9B are plotted on rectangular coordinates. Figure 10B consists of derived curves relating the proportional limit stress in bearing to the ultimate in compression.

Figure 11 is a graphical presentation of the results of repeated-loading tests. Figure 11A consists of a series of consecutive load-deformation curves showing the increase in the apparent proportional limit under progressively increasing loads applied to the same specimen. Figure 11B shows the effect of repeated loadings to the maximum apparent proportional limit (determined in figure 11A) on the total deformation and on the permanent set.

Analysis of Results

Critical Dimensions

Critical values for edge clearance and end margin, determined only for compregs A and C in this investigation, are the minimum values that were found to develop the full bearing strength for each combination of diameter of bolt and thickness of compreg. These values are shown in the last two columns of tables 2 through 14 and are summarized in table 15.

Critical dimensions were based on ultimate stress, as illustrated in figure 2, rather than on ultimate loads as for laboratory-made compreg, because of considerable variations in the measured thickness of one of the commercial compregs (column 5, table 1). Specimens in which the dimension under study was inadequate failed in some manner other than bearing and at a computed bearing stress somewhat less than the true ultimate bearing stress. Specimens, tested under compressive loading, whose dimensions were equal to or greater than the critical dimension usually failed in bearing. Those tested under tensile loading, however, were more erratic in the manner of failure because of the notch sensitivity of the compreg. Many such specimens, particularly those tested at 0° or 90°, failed in bearing and in tension at the bolt hole simultaneously, even though the edge clearance was greater than the minimum that had been found necessary to develop the full bearing strength of the compreg. In all series of tests, however, regardless of the type of failure, a definite dimension was determined beyond which any further increase failed to produce a significant increase in ultimate stress. This value was considered to be the critical dimension.

The critical dimensions, as summarized in table 15, were essentially the same for the three compregs studied, and agreed favorably with those obtained from the laboratory-made material. They were not influenced consistently by the thickness of the compreg within the range studied (1/4 to

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l inch) but were approximately in proportion to the diameter of bolt. It is convenient, therefore, to express these dimensions in terms of diameter, D.

Wherever a difference was found among the critical dimensions for specimens of different grain orientations, those obtained under tensile loading for compreg at 45° were usually smaller and more consistent than those for 0° and 90°. Those obtained under compressive loading at 45° were similarly consistent but were equal to or slightly greater than those for 0° and 90°.

Edge clearances for compressive loading were determined for practically all combinations of diameter of bolt and thicknesses of compreg. While there was some variation among the individual values, there was no welldefined pattern, and the values, in terms of diameters, for all grain orientations were approximately the same. An edge clearance of two diameters was found to be adequate for all combinations.

Edge clearances for tensile loading were determined for only 1/4- and 1/2inch bolts and 1/4- and 1/2-inch thicknesses. For each grain orientation, however, the values exhibited an approximate relationship to diameter, but no consistent relationship to thickness. Thus, the values determined, when expressed in terms of diameter, can be presumed to apply to all combinations of diameter and thickness. The values were found to vary considerably for different grain orientations. Edge clearances of 4, 2-1/2, and 5 diameters were found to be adequate for 0°, 45°, and 90°, respectively.

It should be kept in mind that these values of edge clearance for both tensile and compressive loading were determined by tests employing only single bolts. In fastenings employing two or more bolts in line, the tensile stresses at the critical section are increased proportionately. In wood structural members in which compreg is used for reinforcing plates, tensile and compressive stresses in the plates are reduced by transfer of stress to the wood. The required edge clearances may, therefore, be governed by the shear area necessary to transfer load from the reinforcing to the wood rather than by the tensile or compressive strength of the reinforcing plates.

End margins were determined by direct tensile loading for only 1/4- and 1/2-inch bolts in 1/4- and 1/2-inch compregs. Approximate values for the other combinations were determined by modified compressive loading. Previous tests had indicated reasonable agreement between results obtained by these two methods. Again, the thickness had little effect on the critical dimension. End margins of 5 diameters for 0°, and 90° and 3 diameters for 45° were found to be adequate.

Moisture Content of Compreg

The moisture content of the compreg, determined by oven drying to approximately constant weight, varied from 2.3 to 7.6 percent with an average of 4.3 percent. It is recognized that in drying compreg samples errors are introduced in the indicated moisture content by driving off volatile matter other than moisture. Because of the uncertainty thus created as to the actual moisture content, and since no relationships had been established between moisture content and strength properties of compreg, no attempt was made in this investigation to adjust bearing strength or other values for moisture content.

Bolt-Bearing Strength

Proportional limit loads were determined from load-deformation curves, which were obtained for all bolt-bearing tests. A typical curve is shown in figure 10 of Report No. 1523-A.² The general characteristics of the loaddeformation curves and the method of determining ultimate loads were the same as those described in that report for laboratory-made compreg. Bearing stresses were computed by dividing loads by projected areas.

Typical failures of specimens for each of the four kinds of bolt-bearing tests made in this investigation are shown in figures 3 through 6. Average proportional limit and ultimate loads are listed for each edge clearance or end margin included in each series of specimens shown. The average proportional limit loads and hence average stresses for all five groups shown in figure 3 were approximately the same; all groups were used, therefore, in determining the average bearing stress at proportional limit. In each of figures 4, 5, and 6, however, the group of specimens labeled A developed average stresses at proportional limit lower than any of the other groups in their respective series, indicating that the dimension being varied was inadequate to develop even the proportional limit stress in bearing. Only the last four groups, therefore, were used in determining the average bearing stress at proportional limit for those particular series of tests. The ultimate stresses in each of figures 3 through 6 increased progressively with increase in dimension until the critical dimension was reached, after which they were practically uniform. Only those uniform values were averaged in determining the ultimate bearing stresses for each series. This is illustrated graphically in figure 2.

Bolt-bearing stresses at proportional limit for compregs A, B, and C and at ultimate for compreg A averaged about 20 percent less than those for laboratory-made compreg. The ultimates for both compregs B and C, however, averaged within 5 percent of the laboratory product. Since the composition of compreg A resembles more closely that of the laboratory-made compreg,

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it appears that the influence of composition on the bearing stress is less important than that of other factors not identified in this investigation.

In general, the proportional limit stresses and deformations obtained under tensile loading were from 10 to 35 percent lower for all commercial compregs studied than those obtained under compressive loading. The values of ultimate bearing stress, however, were about the same for both types of loading. It is probable that the lower values for proportional limit stress under tensile loading were due to a difference in end restraint imposed on the bolt by the different types of testing apparatus used. It follows that the proportional limit stresses obtained under compressive loading probably represent more accurately the true proportional limit bearing stresses for the material.

Both proportional limit and ultimate bearing stress decreased with an increase in diameter of bolt for all commercial compregs studied and increased with an increase in the thickness of the compreg. The trends, however, were not as well defined for proportional limit as for ultimate. The principal discrepancies occurred in the two combinations in which the ratio of bearing length or thickness to diameter (L/D ratio) was greatest, namely: 1/4-inch bolt in 1/2-inch compreg and 1/2-inch bolt in 1-inch compreg. The results of previous tests of bearing strength of aircraft bolts in wood parallel to grain indicated that the proportional limit of the wood in bearing is reduced by bending of the bolt if the L/D ratio is greater than one. It is probable that this effect is even more pronounced in compreg because of its greater density and strength and that this accounts for the reduction in proportional limit stress for the higher L/D ratios. There was no corresponding reduction in ultimate bearing stress, probably because none of the L/D ratios employed was sufficiently large to produce such a reduction.

The average values for bearing stress at both proportional limit and ultimate appeared to be unaffected by the grain orientation (fig. 7). The 45° material, however, was more consistent in its manner of failure. Tensile failures in bolt-bearing specimens that developed the full bearing strength were less common in 45° than in 0° and 90° specimens.

Ratio of Proportional Limit to Ultimate Bearing Stress

The effects of unidentified variable factors on the proportional limit and ultimate bearing stresses can be minimized in this analysis by considering the ratios of proportional limit to ultimate. These ratios are shown in column 12 of tables 2 through 14 and are summarized in table 18. While there was some variation among the different compregs, there was a general tendency for this ratio to decrease with increase in diameter of bolt. Except for the two combinations with L/D ratios of 2, there was no apparent effect of thickness of compreg. Excluding these two combinations (1/4-inch bolt in 1/2-inch compreg and 1/2-inch bolt in 1-inch compreg), the average ratios for compressive loading were 0.62, 0.54, and 0.48 for 1/4-, 1/2-, and 1-inch bolts, respectively.

Compressive Strength

Compressive stresses at proportional limit and ultimate are shown in columns 15 and 17, respectively, of tables 2 through 14, and are summarized according to panel numbers in table 1. There was some variation in these values among the different compregs. This variation, however, was only slightly greater than that among individual panels from any one manufacturer. There was no significant variation in compressive strength for different thicknesses of compreg.

The average ultimate compressive stress parallel to face grain for the three commercial compregs (18,000 pounds per square inch) was about 80 percent of that for laboratory-made cross-banded compreg presented in Report No. 1523-A and about 75 percent of that shown in table 2-14 of ANC-18.

Ratio of Bearing Strength to Compressive Strength

Ratios of bearing stress to compressive stress at proportional limit and ultimate are shown in columns 20 and 21 of tables 2 through 14, and are summarized in table 19. Average ratios are derived graphically and tabulated in figure 8. The ratios of ultimate stresses are more consistent than those of proportional limit stresses, probably because of the greater accuracy with which ultimate stresses can be determined both in bearing and in compression. Average ratios at ultimate are shown (on logarithmic coordinates) in relation to thickness of compreg in figure 9A and in relation to diameter of bolt in figure 9B. The extrapolations represented by the dotted portions of the graphs appear to be justified by the general, straightline relationships. Since the compressive strengths were essentially the same for all thicknesses of compreg, these ratios followed the same trend as the ultimate bearing stresses, increasing with increase in thickness of compreg and decreasing with increase in diameter of bolt.

Values taken directly from the graphs of figure 9B are replotted on rectangular coordinates in figure 10A. Five of the seven average test values fall on these curves; the other two values are higher. Curves representing the relationship between proportional limit stress in bearing and ultimate in compression are shown in figure 10B. These curves were derived by multiplying values from figure 10A by appropriate average ratios of proportional limit to ultimate in bearing (compressive loading), shown in table 18.

Results of Repeated-Loading Tests

Only 1/2-inch bolts and 1/2-inch compreg were employed in the repeatedloading tests. Proportional limit bearing stresses determined as a preliminary step averaged 44 and 49 percent of ultimate for tensile and compressive loading, respectively. These percentages are in good agreement with average percentages obtained for the entire series of tests of commercial compreg (44 and 50 percent).

By subjecting matched specimens to loads equal to or slightly greater than the indicated proportional limit, unloading, and then subjecting to progressively higher loads, new load-deformation curves were obtained. In the first few reloadings, the straightline portion continued to a load approximately equal to the highest load applied in the test immediately preceding. The increase in apparent proportional limit persisted until finally a limiting stress was attained at which a break in the curve occurred with each subsequent loading. This is illustrated by the curves of figure 11A. The limiting stresses averaged 58 and 67 percent of ultimate for tensile and compressive loading, respectively, or an average increase of approximately one-third over the proportional limit stress determined by the first loading. For convenience this increased value is referred to as the maximum apparent proportional limit. It is conceivable that, had the tests been continued, the apparent proportional limits might have resumed their tendency to increase after seeming to become stabilized. This possibility will be explored in a separate study of repeated loading.

Other specimens matched to those tested as illustrated by figure 11A were then subjected to 25 loadings to the maximum apparent proportional limit. On the 25th loading, the test was continued to failure. The maximum deformation accompanying the 25th application of the maximum apparent proportional limit load was 0.0160 inch as compared with 0.010 inch at the initial proportional limit obtained in tests on matched specimens. The maximum permanent deformation or set was 0.0033 inch, and the maximum increase in permanent set due to 24 repetitions of loading was 0.0015 inch. These deformations are illustrated in figure 11B. It may be noted that the magnitude of the permanent set had become practically stabilized; hence no appreciable further increase might be expected with further repetitions of loading.

Probable Yield Stress

The bearing stresses at proportional limit indicated in figure 10B are probably unduly conservative for use as yield stresses. The repeated-loading tests, conducted for only 1/2-inch bolts in 1/2-inch compreg indicated that the apparent proportional limit is increased by over-stressing to approximately 1-1/3 times the value indicated in figure 10B, with no significant increase in the permanent set. The maximum apparent proportional limit thus determined might be assumed, tentatively, to represent a suitable yield stress for the material.

Since the lowest average ratio of proportional limit to ultimate bearing stress under compressive loading was 0.48, it can probably be expected that the lowest ratio of maximum apparent proportional limit to ultimate would be 1-1/3 times this ratio, or 0.64. It would seem, therefore, that tentative values for yield stresses in bearing for L/D ratios not greater than 1 might be assumed to be two-thirds of the ultimate bearing strength.

Determination of Design Stresses

The relationship shown in figure 10A permits determination of ultimate bearing stresses for various combinations of diameter of bolt and thickness of compreg on the basis of the ultimate compressive strength. Since the bearing strength is essentially the same for all grain orientations, compressive strength parallel to the face grain can be employed regardless of the grain direction with reference to the bearing load. In the absence of compressive strength data for the particular compreg to be employed, a value of 18,000 pounds per square inch, or approximately 75 percent of that for laboratorymade cross-banded compreg shown in table 2-14 of ANC-18 is reasonably conservative. If it is desired to base design stresses on the results of tests of a particular compreg, only ultimate compressive strength parallel to face grain need be determined. This simplifies the testing procedure, since no deformation measurements are required.

Deformations

The average, maximum, and minimum deformations for each combination of source of material, type of loading, diameter of bolt and thickness of compreg are shown in columns 9 and 11, tables 2 through 14. A summary of all average deformations is presented in table 17.

There was an increase in average deformation at both proportional limit and ultimate with an increase in thickness of the compreg and a decrease with an increase in diameter of the bolt. In general, the deformations at proportional

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limit were smaller for bolt-bearing under tensile loading than under compressive and modified compressive loading. The deformations at ultimate, however, did not show any consistent difference between types of loading.

On the basis of these tests, it could be expected that the proportional limit bearing stress will be attained at a deformation not exceeding 0.025 inch, and the ultimate at a deformation not exceeding 0.10 inch.

Conclusions

1. The average bolt-bearing strength for the three commercial compregs investigated was only slightly less than that of laboratory-made compreg presented in Report No. 1523-A.

2. The average bolt-bearing strength increased with increase in thickness of compreg and decreased with increase in diameter of bolt but was practically the same for grain orientations of 0° , 45° , and 90° .

3. Bearing stresses at proportional limit were reduced somewhat by bending of the bolts in tests in which the ratio of bearing length to diameter of bolt exceeded one; bearing stresses at ultimate were not reduced.

4. Bearing stresses at proportional limit determined under tensile loading were somewhat lower than those determined under compressive loading. This was probably due to a difference in the rigidity of the equipment used. The higher values are probably more truly representative of the actual bearing strength of the material. Bearing stresses at ultimate were essentially the same for both types of loading.

5. The ratios of bearing stress at proportional limit to bearing stress at ultimate decreased with increase in diameter of bolt, but were practically uninfluenced by thickness of compreg, except in those tests in which the ratio of bearing length to diameter exceeded one.

6. The average compressive strength parallel to the grain of the face plies for the three commercial compregs investigated was about 18,000 pounds per square inch. This was about 80 percent of that for laboratory-made compreg presented in Report No. 1523-A, and about 75 percent of that shown for laboratory-made, cross-banded compreg in table 2-14 of ANC Bulletin 18. Compressive strengths were essentially uninfluenced by thickness of compreg.

7. Bearing stresses at both proportional limit and ultimate were related to the corresponding compressive stresses; the relationship between ultimate stresses was the more consistent.

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8. The apparent proportional limit for 1/2-inch bolts in 1/2-inch compreg was increased by over-stressing and subsequent reloading until it became stabilized at about 1-1/3 times its original value. For compressive loading, this was about two-thirds of the ultimate bearing stress. Twenty-five loadings to this stress failed to produce a significant increase in the permanent deformation, or "set," of the compreg. Tentatively, this maximum apparent proportional limit stress might be considered the yield stress for the material in bearing.

9. Ultimate bearing stresses for design for any grain orientation can be determined by applying ratios presented in figure 9A of this report to the ultimate compressive strength parallel to the grain of the face plies. In the absence of specific compressive strength data, a value of 18,000 pounds per square inch is satisfactory. If compression tests of a specific material are made for this purpose, load-deformation readings are unnecessary, since only the ultimate compressive strength is required.

10. Cross-banded compreg can be expected to develop its proportional limit stress in bearing at deformations not exceeding 0.025 inch and its ultimate at deformations not exceeding 0.10 inch.

11. The necessary edge clearances and end margins were essentially the same for the three commercial compress and for the three thicknesses investigated; they varied about in proportion to the diameter of bolt. Except for edge clearances under compressive loading, which were the same for all grain orientations, the critical dimensions for 45° were smaller than for 0° and 90° . Tensile failures at the critical sections in specimens whose dimensions were adequate to develop the full bearing strength were more common in commercial than in laboratory-made compreg. This tendency was less pronounced for 45° grain orientation than for 0° and 90° .

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	2A 3A 4A 5A 47A 47A 47A 410A 11A 11A	1/4 1/4	: .27 : .27 : .27 : .28 : .26 : .26 : .26 : .26 : .26 : .26 : .26 : .26	: .2628 : .2728 : .2628 : .2728 : .2728	24 24 24 24 24 24 24 24 24 24 24 24 24 2	: 24 : 24 : 24 : 24 : 24 : 24 : 24 : 24	1.341 1.330 1.332 1.336 1.356 1.356 1.358 1.358 1.358 1.358 1.358 1.358 1.358 1.358 1.358 1.358 1.358 1.358 1.358 1.339 1.330 1.331 1.331	6,560 6,540 5,930 6,680 6,750 5,700 5,550 5,550 _6,080	26,030 26,010 6,330 5,700 6,820 5,550 6,820	17,460 16,550 16,570 15,760 17,790 517,860 517,860 517,860 15,920 16,290 16,290 16,290	215,700 215,720 15,230 16,180 16,180 16,100
	31A 432A 433A 435A 4		555555555555555555	52- 533 51- 553 550- 552 550- 552 550- 552 551- 555 551- 555 551- 555 551- 555 551- 555 550- 551 550- 551- 550 551- 550- 551	24444444444444444444444444444444444444	24 24 24 24 24 24 24 24 24 24 24 24 24 2	1.332 : 1.337 : 1.341 : 1.337 : 1.333 : 1.333 : 1.344 : 1.349 :	6,76,64,44,000 6,6,6,4,44,000 6,6,6,5,500 7,7,6,6,4,4,5,5,500 7,7,7,9,400 8,4,4,5,5,500 7,7,7,5,4,5,500 7,7,7,5,4,5,500 7,7,500 7,7,5000 7,7,500 7,7,50000000000	: 5,360 : 5,080 : 5,080 : 5,780 : 5,440 : 5,930 : 5,240	15,120 14,650 15,840 17,600 16,720 516,320 516,600 16,570 516,220 17,040 17,780 515,580 515,580 514,400	15,150 14,620 14,940 15,900 16,180 16,180 15,360 15,360 16,170 16,140 15,620
	60A 61A 62A 463A 465A 466A 466A 466A 466A 466A 467A 466A	111111	: .99 : .99 : 1.00 : .98 : .99 : .99 : .99 : .99 : .1.02 : 1.02 : 1.02 : 1.02	1.01-1.02: 1.02-1.03: 1.01-1.02:	24 24 24 24 24 24 24 24 24 24 24 24 24 2	24 : 24 : 24 : 24 : 24 : 24 : 24 : 24 :	1.330 : 1.327 : 1.317 : 1.299 : 1.300 : 1.316 : 1.277 : 1.277 : 1.296 : 1.296 : 1.296 : 1.296 :	6,780 5,620 5,790 6,760 5,750 5,750 5,750 5,760 5,760 5,760 5,760 5,770 5,770 5,770 5,970 5,970 5,970 5,970	5,380 4,920 5,830 6,380 6,460 6,130 5,800 5,800	: 14,470 : 14,140 : 14,250 : 15,970 : 15,370 : 15,350 : 16,740 : 517,470 : 516,270 : 14,760 : 515,660 : 515,900	:
	2B	1/4	29 :	.2930:	24	: 24 :	1,344	6,740		18,430	
в :	ųцв	1/2	•53	·52- ·53:	24	24	1,352	6,770		18,550	
	81B	1	1.04	1.03-1.04	24	24	1.348 :	6,380		19,930	
	122	1/4	.26 .26	.2628: .2527: .2527: .2627:	23 i 23 i	: 81 :	1.359 :	7,770	5,500 : 5,630 :	20,490	: 19,060 : 19,660 : 15,880 : 19,020
C :	10C 11C 12C 13C 14C	1/2	• •57 • • •56 • • •55 •	.4961: .5461: .5457: .5255: .9199:	23 23 23 23	51 51-1/2 51-1/2 51-1/2	1.347 : 1.347 : 1.327 :	6,000 6,050 7,300 7,310 56,310	5,510 5,550 5,390	16,890 17,870 10,570	: 16,380 : 16,480 : 15,420 : 15,570
1	220	1	1.10	1.07-1.10:	23	01-1/2	1.351 :	5,910		19,480	

Table	1Panel	dimensions,	specific	gravity,	and	compressive	strength	of	commercial	cross-banded	1
	com	preg used fo.	r bolt-be	aring test	ts						

 $\frac{1}{Panels}$ 68A, 1B, 30B through 43B, 60B through 60B, 20C and 21C were used for other purposes.

 $\frac{2}{-Based}$ on weight and volume at time of test. Average moisture content.

³/₁₆ 90°-compression specimens were cut from panels 1A, 2A, 4A, and 5A. Values shown were computed from 0° values on the basis of the average relationship for compreg A.

⁴-Panels containing values for both 0° and 90° material. These were the only panels considered in adjusting values for panels 1A, 2A, 4A, and 5A.

 $\Xi_{\rm Mot}$ correlated with bearing tests and therefore not used in tables 2 through 14.

Table 2,--Bolt-bearing strength and critical dimensions for compres A under compressive loading at 0°

International control in the state of the stat		: [autum]	t:					Bolt	Bolt bearing								Compression	sion			11 Rat	Ratio of	in the local data	141 - 24 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141 - 141	1000	
	ter:	panel : thick-:		JmuN :	ber of	specime	ав: : Var1		Proport1 limit	: lano.	Ultim		Ratio of	C : : Numt	: Jer: Variat		ortional imit	: :Ultimat	Ratio	odulu	COMPTOR	to to ssive str	Spg : 880.	i i i	Pur	
		88 88 90 01		: Tote : teste :	al: Tes ad: fo: : propt : tior : 11m1	r fo br-: ul br-: ul bel: me	sted: JT : Lt1-: Lte : ad :			beformer:			tional alluit to ultimate l'	11 80e	c:	Stree 1	s: Deform		: tiona. :limit : :ultimal		:: Propor- :: tional	:Ultimat	ur : 	nnce: ma	retuz	
	Î	(2)	1 1	(#)	.(5)	1 8	<u> </u>		(8)	(6)	(10)	(11)	(12)	1 d	ļ.,	<u>.</u>	<u>.</u>	<u>.</u>	(18)	(61)	(20)	(21)	1		(53)	
	 	Inch							. 8. 1.	Inch :	P. 8. 1.	Inch				P. 8.1	4	1		1,000 p.e.1.			Bol	ers dia	Bolt	
$ \frac{1}{1} \left(\begin{array}{cccccccccccccccccccccccccccccccccccc$		1/4	TA.		ឆ •	• •• •• ••		utmum : 1 rage : 1 thmum : 1		0.0076 : .0096 :		0.0220	0.678	80 ::::::::::::::::::::::::::::::::::::						: 1,788 : 2,193 : 2,421						
$ \frac{1}{1} \left(\begin{array}{cccccccccccccccccccccccccccccccccccc$	10 YO 100 YO	1/2	30A ::					them : 1 trage : 1 them : 2	7.030 : 8.370 : 0.870 :	: 0420.	27.850 29.890 32.940	0927	.615					1		: 1,942 : 2,178 : 2,392		1000	••••			
$ \frac{1}{1} \left(\begin{array}{cccccccccccccccccccccccccccccccccccc$	** ** ** **	1/4							0,340 : 1,770 : 3,210 :		18,640 19,510 20,700	0172	.603	10 THESE &				7 : 13,660 9 : 15,390 • : 20,200		: 1,788 : 2,193 : 2,421	16 E					
		1/2	30A 1					thaum : 1 rege : 1 daum : 1	0,350 : 1,620 : 2,790 :	. 0056 : . 0071 :	20,000 : 21,850 : 25,120 :	0340 0479	-532							1.942 2.178 2.392						
	18 24 24 24 2 N		60A ::					limum : 1 Trage : 1 Amum : 1	2,680 : 4, 230 : 7,000 :	. 0092 : . 0120 .	24, 210 : 28, 240 : 30, 990 :	00629	- 504			** ** **		5 : 15,480 5 : 14,470 5 : 15,200		: 1,994 : 2,306 : 2,658	12					
$ \frac{304}{11} \frac{17}{11} \frac{17}{11} \frac{17}{11} \frac{17}{11} \frac{1}{10} \frac{1}{4} \frac{1}{4} \frac{1}{4} \frac{1}{6} \frac{1}{6}$		•		E	я 			them: 1 trage : 1 them : 1	3, 390 : 4, 600 : 6, 810 :		25,350 27.170 29,700	.0700 .0862 .1080	-537	2 2				44 47 44			R				m	
H/E 114 B F Minimum 10, 750 .0039 22, 490 .0500 .523 138 Minimum 5, 960 .0035 15, 320 1, 757 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 757 1, 236 1, 750 1, 750 1, 750 1, 757 1, 236 1, 714 1, 716 1, 716 1, 716 1, 716 1, 716 1, 716 1, 716 1, 716 1, 716 1, 716 1, 716 1, 717 1, 717 1, 714 1, 714 1, 716 1, 716 1, 716 1, 716 1, 716 1, 716 1, 718 1, 714 1, 714 1, 714 1, 714 1, 714 1, 714 1, 714 1, 714 1, 714 1, 714 1, 714 <th 1,="" 717<="" td="" th<=""><td>** ** **</td><td>4</td><td></td><td></td><td>я </td><td></td><td>10.110.01</td><td>timum : trage : 1 thrum : 1</td><td>9,490 : 0,400 : 1,880 :</td><td></td><td>19,200 20,870 23,660</td><td>04120 0410</td><td>96tt.</td><td>100.005</td><td></td><td></td><td>** ** **</td><td>5 : 13,650 2 : 15,120 3 : 16,450</td><td></td><td>: 1,942 : 2,178 : 2,392</td><td>•</td><td></td><td></td><td></td><td></td></th>	<td>** ** **</td> <td>4</td> <td></td> <td></td> <td>я </td> <td></td> <td>10.110.01</td> <td>timum : trage : 1 thrum : 1</td> <td>9,490 : 0,400 : 1,880 :</td> <td></td> <td>19,200 20,870 23,660</td> <td>04120 0410</td> <td>96tt.</td> <td>100.005</td> <td></td> <td></td> <td>** ** **</td> <td>5 : 13,650 2 : 15,120 3 : 16,450</td> <td></td> <td>: 1,942 : 2,178 : 2,392</td> <td>•</td> <td></td> <td></td> <td></td> <td></td>	** ** **	4			я 		10.110.01	timum : trage : 1 thrum : 1	9,490 : 0,400 : 1,880 :		19,200 20,870 23,660	04120 0410	96tt.	100.005			** ** **	5 : 13,650 2 : 15,120 3 : 16,450		: 1,942 : 2,178 : 2,392	•				
14 14 14 14 10 Maximum 9,560 : 0053 18,210 : 0256 11 2 Average 6,750 : 0026 11,400 1,994 11 501 1,501 1,393 1-1/4 1. 18 19 10 Maximum 10,520 : 0085 122,400 : 0697 1 505 11 2 Average 6,780 : 0033 15,200 1 2,565 11 1,501 1,393 1 1-1/4 1. 19 11 6 Maximum 10,520 : 0066 122,510 0,940 11 1 Maximum 7,160 1 0033 15,200 1 2,656 11 1,5001 1,501 1,5001 1,501 1,5001 1,5001				*1 		 		thread : 1 trage : 1 thread : 1	0.720 : 1.950 : 3.030 :	.0095 : .0113 :	22,490	0280	.523					** ** **		: 1,598 : 2,222 : 2,552	1			8 S	3/1-8	
:: : : : : : : : : : : : : : : : : : :			60A ::					thmum : Trage : 1 thmum : 1	9.580 : 0.180 : 0.520 :		18,210 20,150 22,400	-0256 -0457 -0850	-505					5 : 13,480 5 : 14,470 3 : 15,200		: 1, 994 : 2,306 : 2,656						
	** ** ** **	-	62A ::						8,920 : 9,920 : 0,420 :	.0069 . .0104 .	22,510 : 23,040 : 23,890 :	0440	.431	1				4 : 13.780 9 : 14.250 5 : 15.040		: 1,630 : 1,974 : 2,202					2-1/2	

M Column 8 divided by column 10. 20 Column 5 divided by column 17. 3 Column 8 divided by column 15. 4 Column 10 divided by column 17. 5 End margin for tensile londing determined by modified compressive loading.

Z M 65567 F

Table 3.- Molt-bearing strength and critical dimensions for compress A under compressive loading at 45°

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Matrix Matrix <th matrix<="" th=""><th>bolt ; pene</th><th><pre>Mominal Mominal Panel bolt ! penel inumber! dimenter! thick-!</pre></th><th>1 - 1 0 - 1 - 1 1 - Bus</th><th>i Humber of specimens :</th><th>tetnens</th><th></th><th>Bolt bearing</th><th>1 Jaco</th><th>Ultiante</th><th></th><th>Ratio 1</th><th>I Intio (Bunber of</th><th>of specimens</th><th>sells:</th><th>-</th><th></th><th>Pro</th><th>Propertional limit</th><th>lint</th><th>compression.</th><th></th><th>Ultime</th><th>Ultimate strens</th><th>-</th><th>BALLO :</th><th>Ratio : Modulus of</th><th>of sissticity</th><th></th><th>t-bearing to</th><th>ibelt-bearing stress Critical</th><th>Critical</th><th>diseasions</th></th>	<th>bolt ; pene</th> <th><pre>Mominal Mominal Panel bolt ! penel inumber! dimenter! thick-!</pre></th> <th>1 - 1 0 - 1 - 1 1 - Bus</th> <th>i Humber of specimens :</th> <th>tetnens</th> <th></th> <th>Bolt bearing</th> <th>1 Jaco</th> <th>Ultiante</th> <th></th> <th>Ratio 1</th> <th>I Intio (Bunber of</th> <th>of specimens</th> <th>sells:</th> <th>-</th> <th></th> <th>Pro</th> <th>Propertional limit</th> <th>lint</th> <th>compression.</th> <th></th> <th>Ultime</th> <th>Ultimate strens</th> <th>-</th> <th>BALLO :</th> <th>Ratio : Modulus of</th> <th>of sissticity</th> <th></th> <th>t-bearing to</th> <th>ibelt-bearing stress Critical</th> <th>Critical</th> <th>diseasions</th>	bolt ; pene	<pre>Mominal Mominal Panel bolt ! penel inumber! dimenter! thick-!</pre>	1 - 1 0 - 1 - 1 1 - Bus	i Humber of specimens :	tetnens		Bolt bearing	1 Jaco	Ultiante		Ratio 1	I Intio (Bunber of	of specimens	sells:	-		Pro	Propertional limit	lint	compression.		Ultime	Ultimate strens	-	BALLO :	Ratio : Modulus of	of sissticity		t-bearing to	ibelt-bearing stress Critical	Critical	diseasions
	1 1000		101	al drested	Tester	aristion	1 hell		Strees 1	7.7	proport	Vested 17	Tibeles'	ofeal:	labion-	1.55	1984	-	Deform	ation	1.	+ .0	1		1-Jogor-1	· •0			prestra	atreas	Marancel	Ted 5	
(1) (1) <th></th> <th></th> <th>ł</th> <th>tad: for thropor- thronal i limit</th> <th>tior mate</th> <th></th> <th>Strees :</th> <th>Defor-</th> <th></th> <th></th> <th>tintt i</th> <th></th> <th>8</th> <th></th> <th>L</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>8394</th> <th></th> <th></th> <th></th> <th>tinit:</th> <th></th> <th></th> <th>633 </th> <th>t ratu</th> <th></th> <th></th> <th></th>			ł	tad: for thropor- thronal i limit	tior mate		Strees :	Defor-			tintt i		8		L						8 3 94				tinit:			633 	t ratu				
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	1	ļ	1	1	1 19		11,160 1	0065	22,090 1 22,090 1 21,740 1	6610 6610	065.		1 954			28.8						1.960 F		16.140		2.712				1.769	1-1/2		
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	H 	<u>م</u>	1				13,860 1	0156	85°53	0202	195-	9	9				A 14 14 III II	1				** ** ** ** *	10. mm - 10. mm - 1	15.760						1.1666		2-1/2	
1 3 1 Rations 3,160 055 050 051 051 051 051 051 051 051 151 <td>Į</td> <td></td> <td>1</td> <td><u> </u></td> <td></td> <td>r Minteren 1 Average 1 Hertern 1</td> <td>9,280 : 10,650 :</td> <td>1900. 1900.</td> <td>22.550 22.550 22.550</td> <td>0920- 1190-</td> <td>Ŗ</td> <td>1 14</td> <td>31</td> <td></td> <td></td> <td>** ** ** **</td> <td></td> <td>1</td> <td>1</td> <td>1</td> <td>****</td> <td></td> <td></td> <td>15.340</td> <td>A 10 44 44 1</td> <td></td> <td></td> <td></td> <td></td> <td>1.404</td> <td>2/1-1</td> <td></td>	Į		1	<u> </u>		r Minteren 1 Average 1 Hertern 1	9,280 : 10,650 :	1900. 1900.	22.550 22.550 22.550	0920- 1190-	Ŗ	1 14	31			** ** ** **		1	1	1	****			15.340	A 10 44 44 1					1.404	2/1-1		
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12 10 3 Aurona 10.130 016 12.310 0700 101 13 14.150 0070 101 13 14 150 14.150 1	1	19	1	1		Minimum I Average I Antimum I	8,180 9,600 11,120		21,990	0600	R.H.	10												15.180						1.W9	1-1/2		
11 11 1 1 7 менециа 10,100 0051 22,300 0500 451 11 10 2 менециа 1,500 5,30 5,30 5,30 5,30 5,30 5,30 5,30		6	1		1		10,130 1	810	26.25	0880	105	2	a									in 14 14 14		15.120						1-945-1		2/1-2	
		66	1	ļ					22,910 24,156	0690.	Ę.	#	3	÷ = = = =		÷ = = + +							15,640		86.	2.097		1				s/t-e	

Table 4 .-- Bolt-bearing strength and critical dimensions for compres A under compressive loading at 90°

Math with with with with with with with wi	Womd we 1	: • Kominel	. Banal				Æ	Bolt bearing	8		1				0	Compression	n			Batio of bolt-bearing	of atress	Critical	dimensions
Matrix Matrix<	bolt dinmeter	thick-	number	Nomh	er of sp	ectment	: Variation	: Proport	t ::	e l		Ratio :	: Number	: Variation:			Ultimate:	Ratio : of :	Modulus:: of ::	tr compressi	re stress	Edge :	End. 5
		8 8 9 1 1 1		: Total	d: for ipropor ; limit	for for limited limited limited	5 5		Defor- : mation :			propor- tional limit to ul- timatel	:speci-		3¢rese 1	Defor- mation	17 49 48 49 48 18 18 0 0 19 19 0 10	propor-: tional : limit : to ulz: timatez:		Propor-il tional : limit' :	Itimate ⁴	clearance:	margin-
	(1)		1 1	(1)	: (5)	: (6)	(1)	(8)	(6)	(01)	(11)	1 1		1	(15)	(16)	(11)	1	(19)	(8)	(12)	(22)	(23)
	Inch	Inch						P. 8. 1.	<u>Inch</u>	P. 8. 1.	Inch				1	Inch per			1.000 I			<u>Bolt</u>	Bolt diameters
	T/1	1/1			. 19	13	Minimum Average Maximum	13,860 15,490 18,210	0.0069 .0087 .4010.		0.0220 : .0266 : .0420 :	0.637	<u>6</u> 23	: Minimum : : Average : : Maximum :	62,840 P	60.0030		0.400	<u>6</u> 1, 93ء ::: :::	2.652 :	1.664	1-1/2 :	
	1	1/2	: 36A :	15	. 15	6			. 0063 : . 0093 : . 0131 :	30,510 : 32,020 : 33,570 :	.0400 : .0453 : .0520 :			Minimum : Average : Maximum :	2.760 : 5.080 : 6.720 :		13,980 1 15,900 1 17,500 1	: 61£ •	1,462 :: 1,671 :: 2,559 ::	3.505	2.014	0	
$ \frac{1}{1} \left(\begin{array}{cccccccccccccccccccccccccccccccccccc$		1/1	1	1 2 2 3	18	112			.0058 .0065	18,670 19,630 20,630	.0278 : .0278 :			Average : Maximum		<u>6</u> .0030	ξ14,600 1	- 00 ⁴⁷	61,931 ::	1.985	1.345	1-1/4	
$ \begin{bmatrix} 5/1 & 16 & 16 & 16 & 16 & 10 & katalians \\ 724 & 15 & 15 & 16 & 0 & 036 & 0379 & 25.690 & 0600 & 050 & 17.600 & 0001 & 17.600 & 0001 & 17.610 & 2.790 & 1.761 & 2.794 & 1.761 & 1.971 \\ 724 & 15 & 15 & 16 & 0.056 & 20.53 & 25.690 & 0600 & 0501 & 2.77460 & 0001 & 12.740 & 0001 & 17.610 & 2.790 & 1.701 & 2.791 & 1.910 & 0.910 & 2.910 & 0.991 & 1.910 & 0.991 & 1.910 & 1.9$	6/1	1/2	364	15	115			11, 810 : 15, 920 : 15, 920 :	. 0004 . 00078 . 0090	21,820 1 26,200 1 31,140 1	.0163 .0439 .0620	115.	12	Minimum : Average : Maximum :	2,760 5,080 6,720	.0028	13,980 1	: 61£ ·	1,482 :: 1,871 :: 2,559 ::	2.667 1	1.648	1-1/4	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		-	57A		16	1 10		14,960 1 16,760 1 21,440 1	. 0104 . 0138 . 7150.	27,400 29,450 31,860	.0450 .0576	: 695 ·	12	Hinimum : 7 Average : 1 Maximum :	5.760 6.460 7.360		15.560 16.540 17,690	1 165.	2.157 ::	2.594 :	1.781	1-3/4	
		•	724		: 15		: Minimum : Average : Maximum	12.340 15.040 17.560	.0120 .0163 .0213	23,650 25,880 27,440	0620 07444 0900	. 581	8	Minimum : Average : Maximum :	2,500		12,430	Lott.	1,701 :: 1,910 :: 2,038 ::	2.730	1.910		3
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		c/L :	364	- 20 S	1 12	ъ. 		9,060 9,830 10,580	.0060 .0068	18,730 : 20,300 : 22,230 :	.0240 .0334	, 484 ¹ .	а 	Hinimum : Average : Maximum :	2,760 5,080 6,720		13,980 15,900 17,500	. 61£.	1,462 :: 1,871 :: 2,559 ::	1.935 :	1.277	1-1/4	
67a 16 16 4 warmam 9,780 0003 27,780 00026 17,560 00026 17,650 17,750 17,750 17,175 17,176 17,175 17,176 <			374		12	و 	** ** **	8.280 1 10.220 1 11.000 1	.0084 : .0103 :	21,940 23,200 24,260	.0720 .0797 .0880	, thu	8	Merade : Meximum :	3.510 : 5.080 : 6.540 :		15,050 16,120 17,700	đ.	1, 631 :: 2,012 :: 2,202 ::	2.012	1.434		2-1/2
694 ii 7 5 intimum 10,970 0015 22,300 0700 0732 531 20 areases 6,130 0031 14,640 1,635 1 1,635 1 1,563 6.1 1 5 interment 13,470 .0013 23,260 .0732 .513 20 areases 6,130 .0031 14,640 1,643 1.563 1.563 1 1 1 1 14,640 .0033 15,710 2,023 1.513 1.563 1 1 1 1 14,640 .0033 15,710 2,023 1.563 1.563 1 1 1 1 12,470 .0260 20,470 .0640 .0780 .0171 .456 1.762 1.653 1.563 704 1 1 1 1 1 1 1.666 1.766 1.566 1.563 1.766 1.563 1.765 1.563 1.765 1.563 1.765 1.563 1.776 1.563 1.776 1.565 1.776 1.664	н		674		. 16	9	· ·· ·· ·· ·	9,780 : 11,120 : 12,350 :	*** ** ** *	21,260 : 23,380 : 24,410 :	.0400 .0522 .0600	· 476	12	Minimum Average Maximum	5,760 6,460 7,360		15,560 16,540 17,690	. 391	1,866 :: 2,157 :: 2,270 ::	1.721	1.414	1-1/2	
11 11 6 Marianum 9,220 0.0100 20,470 0.0640 11 Minianum 5,350 .0027 12,950 1.772 11 16 Average 9,550 1.0126 21,550 0.0777 1.456 1.560 1.050 13,610 1.426 1.932 11.698 1.586 1.			₹69	12	-	5	Munimum Average	10,970 11,940 13,470	-0115 -0148 -0200	22, 300 23, 260 24, 090	-0700 -0732 -0780	-513	କ୍ଷ	Minimun Average Maximum	4, 700 6, 130 7, 060		14,040 14,880 15,710	ath.	1,835 2,013 2,428	1. 948 1	1.563		2-1/2
			₹oz		4	9	: Minimum : Average : Maximum :	9,210 1 9,650 : 11,500 :		20,470 : 21,580 : 22,850 :	0060. 1770.			Minimum Average Marimum	5, 350 5, 350 6, 550		12,950 13,610 13,980	. 426	1,772 1,932 2,039	1.698			2-1/2

Acolumn 5 divided by column 10. Acolumn 5 divided by column 10. Acolumn 15 divided by column 17. Jocumn 10 divided by column 17. Boolumn 10 divided by column 17. Dand mergin for teneile loading determined by modified compressive loading. Boo occursion specimens cut from peuel 1A. Values entered ware computed from 0° values on the basis of the average relationship for compreg A.

Z M 65569 F

Table 5.--Bolt-bearing strength and critical dimensions for compreg A under tensile loading at 0°

	•• •		**			4	BOLT DESTINE	60			•				TOTOBO VÁMOO				bolt-bearing	Ing stress	a: Critical	
bolt : panel : dismeter: thick-:	panel thick-:	number	:: Fumber	panel mumber: Fumber of specimens : this thick : :: :: :: :: :: :: :: :: :: :: :: ::	cimens	Variation	: Proportional	tonal	Ultimate		Batio :: of ::]	: Eumber	:: ::Number:Variation:	Proportional		: Bati	9	luboh	:: tompressive	to stress:		Pag
94 47 44 68 88	8 8 9 0	** ** ** ** **	:Total :teste :	::Totkal :Tested :Tested: ::tested: for : for : :: :tropor-: ulti- :: :tional : mate : :: :limit : load :	Tested: for ulti- mete: load			Defor-	Stress 	Defor-	ipropor.:: of : tional ::speci-: limit :: mens : to ul ₁ :: timate*::	: of :speci- : mens			: Defor-	tress propor-: tional : 11mit : to ulz: timate	:propor-: elas- :tional :ticity : limit : : to ul ₂ : :timate ² :	elas- ticity	Propor-: tional	: Propor-:Ultimater : tional : : limit ¹ :		
(3)	(5)	(2)	(f)	: (5)	(9)	(2)	(8)	(6)	(01)	(E)	(12)	. (13)	(17)	(15)	(16)	(21)	(18)	(19)	(30)	(ਸ਼)	(83)	(23)
। मुल्मा	Inch							Inch		Inch				P. 8. 1.	Inch per			1.000			<u>Bolt</u>	Bolt diameters
		****	. 18	. 18	80	Minimum Averaça Marimum	11.670 13.490 16.130	0.0056	27.120 30.840 35.200	0.0280 : .0698 : .1482 :	0.437 :	P	Minimum Average Maximum	5,650	0.0024 .0029 .0033	15.950 : 17.460 : 19.440 :	0-364 :	1.938 ::: 2,157 ::: 2,422 :::	2.121	1.766	m	
	4 1/T		11	11	6	Minimum Average Maximum	15,150	.0038 .0057 .0072	25,080 28,230 31,880	0256	. TL4.	17	Mariaum Maxiaum	3,550 6,550 8,350	0025	12,600 : 15,920 :	64£ •	1, 946 1: 2, 199 :: 2, 384 ::	2.395	1.773	£	
: 	· · · ·		12	15	6	Minimum Averaçe Maximum	11,080 12,670 14,290	.0033 .0046	23,920 25,130 25,830	.0194 .0314 .0350	-50 th	11	Minimum Aversge Meximum	5,550	.0018 .0025 .0029	12,600 1 15,920 1 16,950 1	6f£ ·	1,946 :: 2,199 :: 2,384 ::	2.233	1.579		5
4i		1 1 1	67 .	12	97	: Maruge	12,490	.0085 .0099 .0136	25, 220 29, 190 36, 640	0505	.h28	11 23	Minimum Average Maximum	5,550	.0026 .0031	12,060 : 14,650 : 16,540 :	£#.	1, 847 11 2,096 11 2,376 11	1.924	1.992	2-1/2	
	×	NoH	67	15	#	: Mathum	11,420	.0089 .0105	30.190 32,200 33,500	-0311 -0468 -0625	60 11	33	Minimum Average : Maximum	6,020 7,270 9,100	0027	15,620 1 17,040 1 20,000 1	-H21-	1.773 2.231 2.420	1.612	1.890		5
		34	05	50	~ · · ·	Minimum Average	8,010 9,810 10,620	.0021 .0035	20, 510 21, 790 23, 020	.0138 .0328 .0518	. 450	9	Minimum Average	5,650 6,360 6,940	-0024 -0023	15,950 17,460 19,440	.364	1,938 2,187 2,422	1.542	1.246		
	4	124	19	5	6	: Marimum	9,060 10,130 12,600	94000 94000	19,840 21,130 22,690	0205	6Ltt-	a 	: Minimum : Average : Maximum	3, 340 6,080 7,260	.0018 .0030 .0031	15,200 : 16,290 : 17,460 :	-373	1,881 2,035 2,178	1.666	1.297		
1/2 :		3	15	8	a	Manimum Average	9.720 10.550 12.030	-0038 -0038	19, 380 20, 760 21, 600	.0308 .0308	. 506	9	i Minimum : Average : Maximum	5.360	0025	14.520 1 15.760 1	-376	2, 044 2, 332 2, 332	677.1	1.317		: 2-1/2
4 -		1	ส	12	50 	Minimum Average Maximum	9,410 10,570 12,160	.0039 .0050	21,020 22,840 24,560	.0274 .0387	.1463	53 ::::::::::::::::::::::::::::::::::::	: Minimum : Average : Maximum	5-550 6-1490	0026 1700 1700	12,060 14,650 16,540	.443	1,847 2,096 2,376	1.629	1-559	: 2-1/2	
	N	121	R	13	4	Minimum Average Meximum	7,540 9,220	-0030 -0039 -0039	22,560	Dial	-439	4 	: Minimum : Average Maximum	4,580 5,920	1200.	14.390 15.700	-377	1,820 :: 2,069 :: 2,324 ::	1.557	1.339		3-1/2

Loolumn 8 divided by column 10. Environments divided by column 17

²Column 15 divided by column 17. Joolumn 8 divided by column 15. ⁴Column 10 divided by column 17.

Z M 65570 F

Table 6 .-- Bolt-bearing strought and critical disorsions for compreg A under teasils loading at Hy*

	1000																									and a state of the local division of the loc		THOM TO NOT			
boalist :Sominal:Penel !	at mont	bert Bu	bulk : panel mumber: Fusher of specimum :	theclash		Prepertional	florad	Ultimet.	m te	atte	Sumbar	Satie : Number of specimens	chaomet.	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1000		Froportio	Froportional limit		The second second	: m:	Ultimate stress	10.00	: Batto	ILUDOM 1	Eatio : Modulus of electicity	sticky .		to the		dimenals
1 HANG 1		Aut.	Tutal Tested Tested]	M /Teat				: Stress : Befor-	1 Jefor-	spropor-tEasted.Testedifictal	Tasted.	Tested	Tutal:	-		Strage		ri .	Deformation	-	*0	· % ·	iAverage spropor-1	"propor	•	- 304	:AVILTINGO	8147		-1-10100110001	i margin
			torited for ifor iproper-iulti itiumal imate	<pre># for : for : ipropor-: ulit-: itional : mate : i limit : ical</pre>		1 3%ream 1 20507- 1 1 1 1 1057300 1 1	10101-		TOTAL	1 starts : 1 start : 1 start :				ii	a 0	*95	Avarage	8	8	LAVETAGO				i liati i i liati i i la ul ₂ i				Fruper-	Fropor-:Ultimate ² tional : limite ³ :		
(1) (2)	9 : (3)	(1) (2)	0 1 (5)	(9) 1 (9)	(2)	(8)	(6)	(10)	(TD	(32)	(13)	(10)	1(51)	(91)	(21)	(33)	(13)	8	(12)	(22)	(23)	(12)	(52)	8	1 (22)	(38) :	(63) 1	(%)	(10)	(26) :	(33)
						2.4.1.	tini i	P. s. 1.	Theh						21.5.1	2.6.1. 2.6.V.	7.4.1	Inch per Loch	P. s. 1. Inch per inch per inch per	Inch por	P. c. 1.	P. 2. 1.	P. 8.1.		1.000	1.000 P. E. L.	1.000			1 2014 1	1 <u>Bolt</u>
	ج. مست. د		01 I 01	*	Minimum I Average I Noticue	13,100 15,210 15,870	5100.1 1900.1	26,520 1 28,230 1 29,280 1	0.02%	1 0.538	1E :	ŝ	37 :	Minimum 1 Avoragu 1 Maxteum 1	5,990	0£0.92	ę, <u>xo</u>	0.0027 10031	1-0.0032	20070	19,450	-15.700	1 1 16,120	1 0.331	1.645	1.881	1 2,005	2.40#	162-T	z-1/2	
2 		74 1 3	91 1Q	91 5	Hinimum Aversign Maximum	10, 940 13, 270	10038 10038	868 188	0237	85	N	13	ŝ	Minimu -	5,220	5,560	6,500	¥200. ¥200.	#600. #600.	fico.	15,010	11,180	15,990	Lon.	1.935	1.11	1,535	2,0%2	1.665		tw
l	R	1 10	7 1 14		Miniate Average	12,500 11,660	1900	88.95 89.55 89.55	-0424 -0559 -0680	Ē.	Ş	R	g	Murner Average I	1,580	1,550 7,750 7,550 7,550	5,850	0000 1200 1200	0000	6200'	14,480 16,570 18,000	155.380 16.590 1 16.240	16.500	- A-	1,906	1.724	2,066	2.331	1.92	2.1/2	
a 	₽ 	1 10	1 0	9	Minimus Avarnes Macianus	00.00 10.00 10.00 10.00	-2900 19000	20,450 33,350	0050 1250 1250	655.	3	8	6	Minimum I	1,290 1	0.5.4	6,640	1200 1200	10024 10031	500	15.870	15.240	16,980	166.	10.15 10.15 201.5	1.650	2.047	1.305	1.93		~
5		я 	9	**	I Mutanu I Avarage I Maxtanu	7,750 9,000 10,220	400 6003 6003	80,600 21,310 22,133	0110	1422	18	, P	37 :	Maine -	8.65 9.65 9.65 9.65 1	56.030	6.,90	12001	5.00 JE	268	14,450	A15.700	16.120	166.	2,136	1.88 1	2,006	1,125	1.322	8-1/V	
÷ {		3	15 12	95) 12.222	i Minimus 1 Avernage 1 Maximus	Minimum 1,800 Average 10.070 Maximum 11.570	100 1900	06,750 22,420 24,900	THO.	ŧ	#	9	ត	Minimun Averaga Maximun	24.29	3,910	6,220	式00. 式00.	20030	9(00)	11.790	14,720	16,980	9%. -	525	11,908	2,046	1.619	1.38		01
1		1 126	12 II		Minimum : Average : Maximum :	9,800 10,790	1100 12600 12600	22.123 24.450	-0322 -0482 -0559	The second	5	2.	đ	Mutana : Averaça Karimm	1,670	4,580	5,860	.0080 1200.	6200 9700	6200-	16,570	15,580	16,560	156.	1,906 2,141 2,514	1.128	2,065	1.641	1.479	~	
7		114	9 1 1 1 1 1		i Minimum Arearage	Minimum 1 9,640 1 Average 11,640 1 Maximum 1 13,700 1	2600 2600	.0078 : 24,380 .0060 : 26,260 .0092 : 27,110	0469	Stat.	3	36	83 I	Munimum I	8962	1.030	6,640	1200	1200-1	200.	1780	15, 240 16, 170 17, 180	16,980	165.		2,192 1,500 2,192 1,902 2,146 2,168	2,047	1153	11 day		8-1/8

Joolumn 8 divided by column 19. ¹Column 10 divided by column 25. ¹Go 90° compression specimens and Yerom pennel Ma. Values entered wave computed from 0° values on the bests of the average relationship for compreg A.

Z M 65571 F

*0°
5
loading
tensile
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compres
10
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dimensions
ori tical
and
atrength
bearing
7Bolt
Table 7

-						R		0		Contraction of the local distance of the loc									bolt-bearing stress!	ing stress	orttical	Critical dimensions
Momines Nominationes ::	anel in	umber: :	Number	of spec.	8192	a : :Variation:	Propertional limit	tonal	Ultimate	te e	: Antio :: of ::	: Numb e	:: Number: Variation:		lado	: Ultimate:	Ratio	: Modulus:	:: to to	o stress	Edge	Par
		84 11111	::Total :Tested :Tested ::tested: for : for :: :toncr: ulti- :: :tional : mate :: : limit : load	Tested Tested: for for propor-, ulti- tionel mate 11mit ; load	Tested: for ulti-: mate: lond:		Str	Defor- mation	Stress : Defor- : mation : :		44449	at of of a spect		Strees	Defor-	<pre>stress :propor-: alss- : :tional :ticity : 11mit : : i to ul-: : :timate:</pre>	:propor-: slas- :tional :ticity : limit : : to ul_: :timate ² :		:: Propor-:Ultimate ⁴ :: tional : :: limit ¹ :	Ul tima te ¹	clearance:	margin
	(2)	: (3) :: (4)	(#)	(2)	(9)	: (1)	(8)	(6)	(01)	(11)	(21)	:: (13)	(11)	(15)	(16)	: (21) :	(18)	(19)	: (02)	(12)	(22)	: (23)
Inch :]	Inch :	::::: 					P.e.1.	Inch .	P.e.1.	Inch				P. 8. 1.	Inch per:	P. B. J.		1.000 P1.			Bolt :	Bolt diameters
		::::: A	1 13	fi fi	9 9	Minimum : 11.530 Average : 13,010 Maximum : 15,180	11,530	0.0034 .0063 .0098	23,390	0.0252	0.460	::: 510	Minimum Average Meximum	. 56,010	50.0031	515.720	0.382		: 2.165 :	1.723	3-1/2	
• • • •		94 11	19 19	4	on	Average : Average : Maximum :	: 11.080 : 14.010 : 15.880	.0041 .0058	25,740 27,150 29,200	.0192 .0283 .0283	. 516	6 11	: Maximum : Maximum	5,610	. 0020 . 0032 . 0036	14,840	£££	1,494	2.497	1.613		#
1 1		394 ::	13	2	1	Averace i mumirek	10,100 12,780 14,070	-0072 -0084	28, 550 30, 280 32, 420	.0336 .0511	. lt22	a 	: Minimum : Average : Maximum	4,420 5,440 6,740		12,760	35 ⁴	2,038 :: 2,166 ::	2. 34	1.971		
	1/2 1		 	9	9	Minimum : Average : Maximum :	12,720	0059	33,588	.0440 .0498 .0498		15	Average Meximum	22.250	6200- 1023	14,520	.325	1,606 1,824 2,124	2.750	1.995	#	
				_	~	Minimum : Average : Maximum :	14,790 14,790	.0086	31,200 31,960	(9)	Eén.	15	Average Meximum	3,750	.0023 .0029	14,520	.325	1,606 1,824 2,124	2.523	2.126		5
! 	 	1 	10	9	~	Minimum : Average : Maximum :	7,650 9,060	.0022	20, 360 20, 820 21, 280	-0398 -0422 -0422		012 II	Average Meximum	56,010	5.0031	515,720	.382		1.507	1. 324	5	
		134.11	я 1	я	5	Minimum : Average : Maximum :	10,900 111,960 15,260	.0078 .0055	222 222 222 222 222 222 222 222 222 22	(9)	-5 ⁴ 3	6	Average Maximum	5,270	-0029 10032 14600	14,140	.398	2,005	1.869	1.368	5	
		104	6	6	5	Hinimum : Average : Maximum :	5, 240 9, 140 9, 840	.0012	18,970 20,990 22,820	-0131 -0236 -0369		6	Minimum Average	5,550		13,000 116,100 118,400	345	1.894 :: 1.993 :: 2.092 ::	1.647	1-304		#
	 	398	. 6	10	ñ	Minimum : Average : Maximum :	8,510 10,100 11,560	.0044 .0045	21,520 22,950	.0194 .0207 .0220	69h.	៨ :::::	Minimum Average	5, 740		15,360	読	2,038	1.68.1	1.401	1-1/2	
	1/2 =	11 N		7	ود	Minimum : 10,120 Average : 10,510 Maximum : 11,360	10,120	.0050 .0055	21, 320 23, 070 26, 100	9	.456	1 15	Minimum Average Maximum	4, 200 5,360	1200. 1200.	13,390	Eff.	1,626 11	1.961	11-1471	1 4-1/2	
	4	::::::::::::::::::::::::::::::::::::::	 v	9	4	Minimum : 5,610 Average : 10,230 Maximum : 10,900	8,610 : 10,230 : 10,900 :	.00 ⁴¹	23,533 26,100	9	464.	15	Minimum Average	5,360		15,620	E4C .	1,626 ::	1.909	1.510		1-1/2

-column 5 divided by column 10. Écolumn 15 divided by column 17. Joolumn 8 divided by column 15. Ecolumn 10 divided by column 17. Eno 90° compression specimens cut from panel 54. Values entered were computed from 0° values on the hasis of the average relationship for compreg 4. Eno 90° compression specimens cut from panel 54. Values entered were computed from 0° values on the hasis of the average relationship for compreg 4.

Z M 65572 F

Table 8 .---Bolt-bearing strength for compreg B under compressive loading at 0°

		=				PO.	Solt bearing	ωp							nonpress-on	20				Ing stress
bolt tameter	Nominal :Nominal:Fanel ::	Panel number	Numbe	r of spe	scimena	: -:Variation:	Proportional 11m1t	tonal :	Ultimate		: Ratio :: : of ::	Number	: :Number: Yarlation:	Proportional limit		:	Ratio	wodulu of	:: compressive	ve strags
1	a see		::Total ::fusted ::	::Total :Tostad :Tostad: ::tested: for : for : :: :propor-: ulti-: :: :ticnal : mate : :: : :lunt : load :	: Tor : for -: ulti- : mate : losd		Streau : Defor- : mation	Defor- : mation :	see see	Defor- metion	Defor- :propor-:: of mation :tional ::speci- : limit :: mens : to ul-:: :timate*::	speci-	*****	Stress	Defor- mation	8 8 8 8 8	atress :proport: : tional : : 1imit : : to ulz: : timate :	eles- ticity	Propor-:Ultimate ⁴ :: thomal : :: limit ¹ :	01 timate
(7)	(2)	(3)	(1) ::	(2)	(9)	(2)	(8)	(6)	(10)	(11)	(12)	(13)	: (†1)	(15)	(16)	: (11)	(38)	: (61)	(20)	(2)
Inch	Inch						P. 8. 1.	Inch	P. 8.4.	Inch				P.s.1.	: Inch per	P. S. L.		1.000 : 2.2.5.1		_
	1/4	8			. 10 	Minimum : Average : Maximum :	16,460 : 18,040 : 19,340 :	0.0084 : .0097 : .0124	27,420 26,690 30,040	0.0310 	1 0.629	9	Minimum : Average : Maximum :	5, 840 6, 740 7, 970	0.0028 .0034	16,350 18,430 19,780	0.366	1,545 1,987 2,162	2.677 :	1.557
1/1	1/2	1 (B)H1	1 12	1 12	1 12	Minimum : Average : Maximum :	22,600	.0112	34,200 38,100 40,680	00110	.593	18	Minimum : Average : Maximum :	5,770 6,770 7,920	.0026 .0032 .0036	15,720 18,550 19,950	- 365	2,008 2,152 2,280	3-338	2.054
	1/14	8	-	1		: Minimum : Average : Maximum :	11, 310 :	. 0070 . 0076	23,270 24,140 24,860	-0450 -0517	н 16ң н	10	Minimum : Average : Maximum :	5,840	.00140	16,350	• 366	1.987		1.310
1/2	1/2	: Ehti :		1		Minimum : Average : Maximum :	14,980 : 16,650 : 13,140 :	.0103 .0113	28,400 29,780 31,260		-559	18	MAnimum : Average : Maximum :	5.770	.0026 .0032 .0036	15,720 18,550 19,950	• 365	2,152 2,152 2,280	2.459	1.605
	1	EIS .	9	9		Minimum : Average : Maximum :	14,260 15,040	.0110 .0157	34.740 38.130 39.580		9th.	10	Marimum : Marimum : Maximum :	5.500	.0022 .0027 .0033	18, 870 19, 930 22, 560	.320	2,186 2,414 2,521	2+357	1.813
	1/2		6	6	6	Minimum : Average : Maximum :	12,450	.0085 .0093 .0093	22, 880 24, 440 21, 240		. 535	18	Minimum : Average : Maximum :	5,770 6,770 71,920	.0026 .0032 .0036	15,720 18,550 19,950	.365	2,005 2,152 2,280	1.931	1. 318
-			80	80	80	Average	11,670	.0108	27,140 28,520 30,000		11 0/4-	10	Manimum : Average : Maximum :	5,500 6,380 7,170	.0022 .0027 .0033	18,870 19,930 22,560	82.	2,186 2,414 2,521	2.100	1.431

Acolumn 8 divided by column 10. Acolumn 15 divided by column 17. Jcolumn 8 divided by column 15. Loolumn 10 divided by column 17.

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Table 9.---Bolt-bearing strength and criticul disensions for compres C under compressive loading at 0°

'	".	:				Å	Bolt bearing	9				::		,	Compression				Batio of bolt-bearing	ne stress:	Critical	:: Ratio of : ::bolt-bearing stress: Critical dimensions
bolt : Manager:	bolt : panel :r dismeter: thick-:	Moninal Hominal:Fould ::	Humber	beds jo	imens	Variation	: Proportional	t	Ultimate	ete :	: Batio :: of ::	:: Number	:: :::::::::::::::::::::::::::::::::::	Proportional		: Rati Ultimate: of	Ratio : of :	: Batio : Modulus:: : of : of ::	to compressive	:: tompressive stress:	S MA	End 5
			::Total :Tested ::tested: for :: :: :: :: :: :: :: ::	- 1	. Tested: for . ulti- . mate . load		2440 89	Defor-	: Stress : Defor- :propor-:: of Defor- : metion :linus ::pec: metion : : : to ui-:: : : to ui-:: : : tilmstef::	Defor- : mation : :	:propor-:: of :tional ::speci- : limit :: mens : to ul-:: timatet::	:: of :: speci- :: mens		244 244 255	Stress : Defor- :	<pre>: stress :propor-: elas- : tion1 :ticity : iimit : : to ulp: : timatef:</pre>	:propor-: elas- :tional :ticity : limit : : to ulz: :timate2:		:: Propor-:Ultimate :: tional : :: limit ¹ :	Ji timate	mater : clearance :	Dargin-
6	(2)		(11)	(2)	(9)	(2)	(8)	(6)	(10)	(n)	(12)	(53)	(14)	(15)	(91)	(11)	(18)	(19)	(20)	(21)	(22)	(23)
Inch	Inch	# = = 	 				P.8.1.	गुरुष	P. e. 1.	Inch				5 e.1.	Inch per	P. 6.1.		1.000 P. e. l			: Bolt :diameters	Bolt dismeters
	1/t	10 10	8	8	15	Minimum : 17,280 Average : 18,350 Marimum : 19,490	17,280 18,350 19,490	0.0062	30,100 32,890 34,140	0.0360	0.558	36	Minimum Average	5,000 7,230 9,220	0.0014 : .0033 : .0052 :	15,960 18,740 22,370	0.386	1,646 :: 2,104 :: 2,330 ::	2.538 :	1.755	¢4	
	1/2	100	15	11	12	Minimum : 17,360 Average : 19,740 Maximum : 21,160	17,360 19,740 21,160	0086	35, 240 36, 240	.0280 .0439 .0620	• 545	<u> </u>	Minimum Average Maximum	4, 860 6,000 7,800	.0026 .0033 .0043	13,200 16,160 19,920	-371	1.476 1.843 2.405	3.290	2.243	1-1/2	
4 	1/4	10 10	8	8	12	Minimum Average Marimum	14,250 14,250 16,760	.0048 .0061	24, 920 27, 240 28, 940	0769	.523	36	Average Maximum	5,000 7,230 9,220	.0014 .0033 .0052	15,960 16,740 22,370	- 386	: 1,646 :: 2,104 :: 2,330 ::	1.971	1547-1	1-1/2	
1/2	1/2	100	15	5	6	Minimum Average Marimum	12.180	.0102	26, 920 29, 180 33, 700	.0290 .0340 .0430	.561	3	Minimum Average	4,860 6,000 7,800	.0026 .0033 .0043	13,200 16,160 19,920	-371	1,476 1,843 2,405	2.728	1.806	3/1-1	
****		520	9	9	9	Minimum Average Maximum	14,440	.0132 .0153 .0166	33, 540 36, 240 39, 000	0200.0000	• 398	12	Minimum Average Meximum	5,220 5,910 6,510	.0025 .0028 .0032	17,580 19,480 20,620	903	2,005 :: 2,136 :: 2,458 ::	2.443	1.860	Bear	: Bearing only i
****		1007		7	6	Minimum Average Maximum	9,100 10,820 12,840	.0051 .0066	22, 810 23, 710 24, 770	0247 0347 0560	.456	8	Murimum Average	4,860 6,000 7,800	.0026 .0033 .0043	13,200 16,160 19,920	-371	1.476 :: 1.843 :: 2,405 ::	1.803	1.467	1-1/2	
	: : : : 2/1	110	12	12	6	Minimum Average Maximum	12,020 13,420 14,660	-0102 4210	24-540 29-900	.0520 .0734	06t	ð.	: Mininum : Average : Maximum	5,390 6,850 8,250	.0026 .0033 .0041	14,170 16,890 19,260	904	1,700 2,099 2,476	1.959	1.622		2-1/2
.		22C :: :	10	10	10	Minimum Average Marimum	12, 340 13, 400 15, 070	.0113 .0121 .0132	26, 730 28, 230 29, 960	.0580 .0685 .0620	-tr	51 11 15	Minimum Average Maximum	5,220 5,910 6,610	.0023 .0026 .0032	: 17,580 : 19,460 : 20,620	-303	2,005 :: 2,136 :: 2,458 ::	2.267 :	6 111 .1	t Bear	Bearing only
10 10 10 10	divided divide divide	^A column 5 divided by column 10. ² Column 15 divided by column 11. ³ Column 6 divided by column 15. ⁴ Column 10 divided by column 17.	m 10. m 17. m 15. m 17.			Acolumn 8 divided by column 10. 2column 15 divided by column 17. 3column 8 divided by column 15. 4column 10 divided by column 17.					_		-	-								

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Table 10.--Bolt-bearing strength and critical dimensions for compres C under compressive loading at 45°

to to lowerstand	to the strengt strengt bigs : Bad	4	an 1	tt ² 31) : (32)	1 (T	mte ¹ 31) (32) 2011 11meter	mte ⁴ 31) (12) 1 2201 590 2 11 12 12 12 12 12 12 12 12 12 12 12 12	11 (12) (12) (12) (12) (12) (12) (12) (1	2.604 1.790 2 2.604 1.590 2 2.771 2.172 2 2.042 1.470 2 2.042 1.470 2	(1)(1,100,142 (3)) (3)) (3)) (3)) (4)) (4)) (4)) (4))	(1): (1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,
Hatfo i Modeline of electrolity .	compressive Average		Contraction of the second	(62) :	(23)	(29) 1,000 1,000 2,051	(29) 2,024 2,024	(25) 14,000 2,024 1,630 1,630	(53) 1,000 2,024 2,024 2,024 1,610	(29) 1,610 2,024 2,024 1,610 1,610	(2) (2) (2) (2) (2) (2) (2) (2) (2) (2)
	8		1 (26)	and a second sec	1. 2.000	+	1.176 1.176 1.176 1.176 1.176 1.176 1.176	1.000 1.000	1.111 1.1111 1.11111 1.111111	1000 14000 1	4 2.132 4 2.132 4 1.1956 4 1.1956 4 1.1956 4 1.1956 4 1.1956 4 1.1956 4 1.1956 5 2.135 4 2.1354 5 1.1776 5 1.1076 5 1.106 5 1.006 5
	proper-1 0"		(13) · (51)	二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十二十	1.000	0.350 2.10 2.10 2.10 2.10					1,120 1,
	-rodord: sperext:	1248	(25) : (26)	- 1	1.1.1	· · · · · · · · · · ·				6.270 6.270 6.270 6.270	6.270 6.270 6.270 6.270 6.270
	41 .06			1 (12) 1	1.1.1	15,860 19,066	21,180 15,380 15,380 15,380 15,380 15,380 15,380 15,380 15,380 15,380 15,380	C.4.) (24) (24) (24) (24) (24) (24) (24) (24	(24) 	(24) E. E. L. (24) E. E. (25) E. (25) E	(24) (25) (25) (25) (25) (25) (25) (25) (25
	•0		(23)		777	15, %0 18, 70 22, 370	15, 960 15, 960 18, 790 22, 570 15, 160 15, 980	15, 960 15, 960 16, 96	L 15, 200 15,		
	ġ	Averses	(22)	And the second s	auch per lack per F.s.i.	er Lack per	sc lack per lack per	active interviewe	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	rudath 201 aground 201 2010-000-000-000-000-000-000-000-000-000	404.00 40.00 1
	Reformation	8. 	(12) : (-							
Propertional limit.		*0 •2	(20)	ł	· [4] - ·	0.0014 0.0014 0.00152	4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	20 00014 00014 00014 00014 00014 00014 00014 00014 00015 00015 00015 00015 00015 00014 00000000	00 00055 00 00055 00050 00055 00050 00050 00050 000050 00050 00050 00050 00050 00050	Limit Entration 1.1 Limit Entration 1.1 Limit Entration 0.0013 0.0013 0.0013 0.0015 0.0013 0.0015 0.0013 0.0015 0.0013 0.0015 0.0013 0.0015 0.0013 0.0015 0.0014 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015 0.0015	1.1 [bah, point 1.1 [b
TOTOTA 1		90° : Average	(16) : (17) : (18) : (19)	allow a house	P. 5. 1. 2. 8.	240 6,65	240 240 000 6,65 250 6,67 2510 5,97	R.e.1. R.e.1. R.e.1. 7.200 6.620 7.250 6.620 7.250 6.530 5.300 6.620 7.200 6.620 7.200 6.620 7.200 6.620 7.200 6.620	R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e.1. R.e	2.1. 3.1. 3	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1
	Strees	8. 	(11) (1)		1-1-1-2	2.200 5. 1.230 6.	2200 5- 1,230 6- 1,230 6- 1,230 7, 1,860 4-	4 1000 0 0 1000 0 0 1000 0 0 1000 0 0 1000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Earl Earl Earl Earl Earl Earl Earl Earl 7.200 5.000 5.000 5.000 7.200 5.000 5.000 5.000 7.200 5.000 5.000 5.000 7.200 5.000 5.000 5.000 7.200 5.000 5.000 5.000 7.000 5.000 5.000 5.000 9.200 5.000 5.000 5.000 7.000 5.000 5.000 5.000 7.000 5.000 5.000 5.000 7.000 5.000 5.000 5.000	E.B.1.1 E.B.1.1 E.B.1.1 E.B.1.1 7.200 5.000 5.000 5.000 9.201 7.200 5.000 5.000 9.203 7.200 5.000 5.000 9.203 7.200 5.000 5.000 9.203 7.200 5.000 5.000 9.203 7.200 5.000 5.000 9.203 7.200 5.000 5.000 7.200 7.200 7.200 5.900 7.200 7.200 7.200 5.900 7.200 7.200 7.300 5.900 7.200 7.200 7.300 5.900	F. Li F. Li F. Li F. Li 7. 200 5. 200 6. 600 9. 200 7. 200 6. 600 9. 200 7. 200 6. 600 9. 200 7. 200 6. 600 9. 200 7. 200 6. 600 9. 200 7. 200 5. 900 6. 4000 7. 200 5. 900 7. 200 7. 200 6. 600 7. 200 7. 200 6. 600 7. 200 7. 200 6. 500 7. 200 7. 200 6. 500 7. 200 7. 200 6. 500 7. 200 7. 200 7. 900 7. 200 7. 200 7. 900 7. 200 7. 200 7. 900 7. 200 7. 200 7. 900 7. 200 7. 200 7. 900 7. 200 7. 200 7. 900 7. 200 7. 200 7. 900 7. 200 7. 200 5. 900 8. 2. 200 5. 200 5. 900 <
	d.Total:	L	1161			E. F. L. E. E. L. E. E. L.	Z.B.1.1 Z.B.1.1 Z.B.1.1 Z.B.1.1 Z.B.1.1 Rational 9.000 5.300 6.600 6.600 Maximum 9.200 7.300 6.000 6.600 Maximum 9.600 6.600 6.600 6.600 Maximum 9.200 7.300 5.900 5.900 Maximum 9.600 9.500 5.900 5.900 Maximum 7.800 7.300 7.900 5.900	Annual Control Control Mathema 5,200 5,200 Mathema 5,200 5,200 Mathema 5,200 5,200 Mathema 5,200 7,200 Mathema 1,520 7,200 Mathema 1,650 7,200 Mathema 1,650 7,900 Mathema 1,650 7,900 Mathema 1,650 7,900 Mathema 7,500 7,900	Minimum Minimu	Mithama 5 Arons 1 Arons 1 Aron	Management of the second secon
	ed. Total :			11((1) : 1							
O TO VOINT	limit : Stress : Defor : proportifested: fested: fotal	mation float as UrstyUr 1 limit : 1 1 to ul-: : : : 1 timate ¹ : : : :	(11) : (12) : (13) : (14) : (15):	Contraction and a second		8	x 3	92 93 92	× 8 × 8	% 3 % 3 3	× & × & d
· Derio · Do	i of proper different of specimens	: limit : 1 % ul_: 1 % ul_:	(12)			0.574					
	1 Defor : prop		(11)	मुरम		ō · ·	0.0740	04/20 24/20 20/20	0.0710	0440 04450 04450 04450 04450 04450 04450 04450 04450 04450 04450 04450 04450 04450 04450 045000 04500 04500 045000 04500000000	0440 0440 0440 0440 0440 0440 0440 044
· TI + in the	1 30 ALL		(01)	P. 8.1.		2001	884 48E	222 222 222	284 488 828 488	2021 1222 2020 1288 101 122 122 1220 1288	107 KAX X28 488 868
Constant,	opertional	: Stress : Defor- i mation i i	(6)	Tari inch		0.0069	0.0069				
- Business	t Proportional lon: limit		(8)	1 2.6.4		Minimum : 15, 440 : 0.0069 : 4verage : 17, 240 : .0083 : 4verage : 19, 940 : .0083 : 105 :	Ministeren 15,440 Averaça 11,240 Maximum 19,940 Miniumum 14,690 Averaça 16,570 Maximum 16,700	Milaiana 15,400 Average 17,200 Average 13,500 Milaiana 14,600 Maciana 14,700 Maciana 11,500 Maciana 11,500 Maciana 11,500 Maciana 13,500	Matheman 15, 400 Matchman 15, 400 Matchman 19, 500 Matchman 14, 650 Matchman 11, 300 Matchman 13, 550 Matchman 10, 700 Matchman 10, 700 Matchman 10, 700 Matchman 17, 100	Matchine 15, 300 Matchine 17, 100 Matchine 9, 700 Matchines 17, 100 Matchines 17, 100 <t< td=""><td>Matakene 53,400 Matakene 137,800 Matakene 137,800 Matakene 136,570 Matakene 131,520 Matakene 132,500 Matakene 132,500 Matakene 132,500</td></t<>	Matakene 53,400 Matakene 137,800 Matakene 137,800 Matakene 136,570 Matakene 131,520 Matakene 132,500 Matakene 132,500 Matakene 132,500
and a	: Number of specimens : 		(2)		1						
	ted :Test	i for i for i ipropor-: ulti-: itional : mate : i limit : load :	(2) 1 (6)	(a) (a.		30 : 12	 				
	Total :Tested :Tested	ttestedi for : for : :propor-: ulti-: : tional : mate : : linit : losd :	(4) : (5)			 8					
	nuber		(2) 1 (3) 1 (4)			: IC :					
WETTERS 1	bolt : penel : diameter: thick-: : ness :		1	Inch Inch	₩/T	4	1/4 1/2				

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Table 11.--Bolt-bearing strength and critical dimensions for compres C under compressive loading at 90°

÷

	<pre>1 1 1 Wominal :Nominal:Panel :: bolt : namel :number: Number of specimens</pre>	1: 1. Number of specimen	ber of specimen				Bolt bearing	6 formal :	Utimate	ata .	Batto			Compre	Compression	a	Untio -1	Batto Modulum.	Eatio of bolt-bearing	o of ing stress:	:: Estio of : -::bolt-bearing stress: Critical dimensions	dimensions
	diemeter: promet :	:::?erlated .Tested.	1 .Tested .Tested.		: Variati	HO					ii jo	Number: 7	Tarlation:			Ultimate:	ot	of 11	compress11	re streas	Itige :	Bad. 5
	::tested: for : for : :: :toroor: ulti-: :: :tionel: mate : :: : itinit : load :	::tested: for : for :Propor-: ulti-: :: :tional : mate : :limit : load	ed: for : for : propor-: ulti-: tional : mate : ilmit : load :	for r-: ulti- 1 : mate 1 : load			89994 9994 9994 9995 9994 999 999 999 999		· · · · · · ·		tional :: limit :: to ul-:: timatok::	apeci-:		Stress :			tional : limit : to ulz; timatez;	ticity ::	Propor-:(tional : limit.	J. tiaste		- TRIE
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$: (3) :: (4) : (5) : (6) : (7)	(5) : (6) :	(5) : (6) :	: (9)		1 1	(8)	(6)	(01)	1 1	1 1	(13) :	(17)	(15)	(91)	;	1	÷	1	(a)	(22)	(23)
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			• • • • •	• •• •• •			1-8-A	<u>Inch</u>	70-8-1-	Inch	: = = :		•••		Inch per inch			1.000			Bolt diameters	<u>Bolt</u> dlameters
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	IC :: 20 : 20 : 12 : Meanum Maximum	20 20 12	20 12	12			15,980 17,840 19,240	0.0056 .0079 .0103	28.980 30.710 32.400		0.581 ::	8	Minimum : Average : Maximum :	5, 240 : 6,000 : 7, 250 :	0.0028	15,880 19,060 21,180		1.766 :: 1.945 :: 2.132 ::		1.611	~ ~ ~ ~ ~	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	10C :: 15 : 15 : 9 : Average	15 15 9	6	9 Average	Mariana Mariana		15,700 16,820 18,200	.0080 .0092	30,680 32,640 35,330	.00400 .0577 .0800	.515	4	Minimum Average	4,510	7200- 145.00-	13,180 16,380 19,080	.364	1,174 :: 1,777 :: 2,164 ::	2.617	1.993		
-0091 23,060 .0400 .44,510 .0024 13,150 1.174 2.186 1.518 11/2 -0103 28,450 .0472 .555 44 Averages 5,970 .0024 15,380 .364 1.174 2.186 1.518 11/2 -0126 28,420 .0540 14 Averages 5,970 .0042 13,180 1.174 2.156 1.518 11/2 -0054 28,420 .0540 14 Averages 5,970 .0027 13,180 1.174 2.156 1.518 11/2 -0054 28,120 .0734 15,180 1.174 1.558 1.590 1.3/4 .0055 24,400 .0860 .415 14 Average 5,970 .0024 19,080 .564 13/4 .0012 25.6700 .0690 .415 .4500 .0025 14,140 1.518 13/4 .0012 25.700 .0025 14,140 .1510 .0025 14,140 1.518 .0128 25,020 .06340 .364	IC :: 22 : 22 : 13 : Marinem :: 22 : 22 : 13 : Merrage	22 22 13 1	22 13 1	1 13 1	: Minimum : Average : Maximum :		12.860 : 14.930 : 16.700 :		25.350 27.980 31.300	0195		8	Minimum : Average : Maximum :	5,240	- 0028 - 0028 - 0035	15,880 19,060 21,180	.315	2,132 ::	2,458	1.468	1-1/2	
0054 20.720 0.0580 11.77 1.536 1.174 1.556 1.576 0068 22.120 0.714 14 Average 5.970 0.004 13.126 1.174 1.556 1.576 1.374 0.0022 22.120 0.0540 15.970 0.004 19.060 2.164 1.556 1.570 1.374 0.0021 22.100 0.0660 19.600 2.14,900 2.144 1.556 1.374 0.0012 22.870 0.0540 Maximum 4.500 0.022 14,340 1.4420 1.518 0.011 22.870 0.0540 Maximum 4.500 0.022 14,340 1.7792 1.518 0.0151 27,420 0.0540 Maximum 5.600 0.041 2.022 1.91799 2.164	100 :: 15 : 14 : 9 : Marimum	15 i 14 i 9 i	1 14 1 2 1	: Minimum : 9 : Average : Maximum	Minimum Avarage Maximum				23,060 24,870 28,420	.0400 .0472 .0540	-585 ::		Marage : Marage : Marimum	4,510 5,970 7,280	1200- 14500-	13,150 16,380 19,080	- 36	1.174 :: 1.777 :: 2.164 ::	2.186 :	1.518	1-1/2	
. 0101 : 22.870 : .0540 : :: : MAXIMUM : 4.500 : .0056 : 14.740 : : 1.442 :: : .0128 : 25.020 : .0831 : .476 :: 54 : Average : 5.510 : .0032 : 15.480 : .334 : 1.709 :: 2.160 : 1.518 : : .0161 : 27.420 : .0960 : :: : : : : : : : : : : : : : : : :	10C : 14 : 14 : Avarage Avarage : 14 : 5 : Avarage	14 : 14 : 06	14:15 Minianum Average Maxianum	: Minimum : 8 : Average : Maximum	. Minimum : Average : Maximum		7,170 1 9,180 : 11,180 :		20,720 22,120 24,420	.0580 .0714 .0860	. ⁴¹⁵ ::		Minimum : Average : Maximum :	4, 510 : 5, 970 : 7, 280 :	-0027 -0034 -0042	13,130 1 16,380 1 19,080 1		1.174 :: 1.777 :: 2.164 ::		1.350	1	
	110 :: 15 : 15 : 9 : Marimum : 10,710 :: 15 : 15 : 9 : Marimum : 11,200	15 15 9	15 9		Minimum Average Maximum		10,710 11,900		22,870 25,020 27,420	.0540 .0531 .0960		ま	Minimum Average Maximum	4,500 : 5,510 : 6,690 :	.0026 .0032	14, 340 16, 480 20, 480		1,709 :: 2,022 ::	1	1.516		2-1/2

Column 8 divided by column 10.

Roburnm 15 divided by column 17. Roburnm 8 divided by column 17. Roburnm 10 divided by column 17. Sand margin for tensils loading determined by modified compressive loading.

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Table 12.--Bolt-bearing strength and critical dimensions for compreg C under tensils loading at 0°

Num Mant Mant <th< th=""><th></th><th></th><th></th><th></th><th></th><th></th><th>Ŕ</th><th>Bolt bearing</th><th>26</th><th></th><th></th><th></th><th>:: :</th><th></th><th>-</th><th>Compression</th><th>ac</th><th></th><th>::'</th><th>Eat1</th><th>o of</th><th>: Critton</th><th>dimonetone</th></th<>							Ŕ	Bolt bearing	26				:: :		-	Compression	ac		::'	Eat1	o of	: Critton	dimonetone
Math Final (1) Fin	bolt :	panel thick-;	redaunt	Kumbe	r of spe	cimens	Variation		tonal	Ulth		Batio :	: Number	: Variation			Ultimate:	Batio of	Modulus:	t.	ve stress	e3 MI	मृष्य
				Total	Tested for propor- tional	for for ult1- mate load		Stress	Defor- mation	Stress		propor-: tional : limit : to ul-: timstel:	: of :speci- : mens	** ** ** ** **	Straes			propor- tional limit to ul- timate		Propor-il tional : limit	D1 timate		
$ \frac{1}{10} $	5		1	E	1	(9)	(2)	(8)	(6)	(01)	(E)	1	: (13)	(111)	(15)	(31)	(11)	(18)	i 1		(12)	(22)	(23)
	- qou	Tuch						B.8.1	Inch	P. 8.1.	Inch					Inch per			1.000 P. B. L.			: Bolt dismeters	
$ \frac{1}{16} \frac{1}{26} \frac{1}{10} \frac{1}{10}$		4	S 20 20	F	16	6	Minimum Average Maximum	13,490 17,060 19,140	9600. 1700.0	33, 350	0.0425	0.512		Minimum Average Maximum	3.560	0.0017	18,220 20,490 23,340	162-0	2, 295 :: 2, 295 :: 2, 720 ::	2.801 1	1.628	5-1/2	
		T/#	8	9	9	9		17,560	1900. .0087	31,310	0360		1	Minimum Average Marinum	3.560	-0027 -0027	18,220 20,490 23,340	. Tes.	1.555 :: 2.295 :: 2.720 ::		1.528	- Fiz 20 00	*
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	*		120	1	52	72		11,560	.0086 .0119 .0184	32,550 35,580 39,610	.0550 .0693			Minimum Average Maximum	6,150 7,300 9,000	.0027 .0033 .0042	17,870	604	1.753 :: 2.192 :: 2.502 ::		1.991	2-1/2	
		2/1	120	1	53	198		12,000 15,290 19,060	- 0072 - 0105	33,700 36,910	.0600			Minimum Average Maximum	6,150 7,300 9,000	.0027 .0033	17,870	60t-	1.755 :: 2.192 :: 2.502 ::	2.095.1	2.065		ㅋ
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$			8	fi	12	-		8,060 9,930 12,340	.0036 .0036	20, 420 26, 290 26, 290	.0186	Étt.		Minimum Average Maximum	3.560	.0027 .0027	18,220 20,490 23,340	762 .	1,555 :: 2,295 :: 2,720 ::	1.631	1.094	F1/2	
1/2 Mithiama 6, 100 0021 17, 870 0.09 1, 757 1, 422 1, 571 1/2 1 1 1, 870 000 2, 566 1, 51 1, 51 1, 60 1, 757 1, 422 1, 51 1/2 1 1 1, 870 000 004 2, 500 1, 51 1, 51 1, 52 1, 42 1, 51 1/2 1 1 1 1, 50 000 004 2, 50 1, 51 1, 51 1, 52 1, 52 1, 51 1, 52 1, 51 1, 52 1, 52 1, 51 1, 52 1, 52 1, 51 1, 51 <t< td=""><td></td><td>*/</td><td>2</td><td></td><td>5</td><td>2</td><td>Marianum Marianum</td><td>9,490 11,520 13,460</td><td>-0033 -0043</td><td>22,800 24,890 27,520</td><td>Dial</td><td></td><td>1</td><td>Minimum Average Marimum</td><td>3.560</td><td>.0017 .0027 .0050</td><td>18,220 20,450 23,340</td><td>162.</td><td>2,295 :: 2,295 :: 2,720 ::</td><td></td><td>1.215</td><td></td><td>1-1/2</td></t<>		*/	2		5	2	Marianum Marianum	9,490 11,520 13,460	-0033 -0043	22,800 24,890 27,520	Dial		1	Minimum Average Marimum	3.560	.0017 .0027 .0050	18,220 20,450 23,340	162.	2,295 :: 2,295 :: 2,720 ::		1.215		1-1/2
120 :: 20 : 20 : 11 Average :11,400 : .0046 : 22.120 : .0460 : .147 :: 54 : Average : 7.300 : .0077 : 17,770 : .1.773 :: 1.562 : 1.397 : .1.577 : .44 : Average : 7.300 : .0073 : 17,570 : .409 : 2.132 :: 1.562 : 1.397 : .1.510 : .1.510 : .2.502 : .2.502 : .1.562 : 1.397 : .1.510 : .2.502 : .2.502 : .1.562 : 1.397 : .1.510 : .2.502 : .2.502 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.300 : .0042 : 21,120 : .2.502 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.397 : .1.562 : 1.300 : .0042 : 21,120 : .2.502 : .1.562 : 1.397 : .1.577 : .1.500 : .0042 : 21,120 : .2.5762 : .1.576 : .1.5777 : .1.577 : .1.577 : .1.5777 : .1.5777 : .1.5777 : .1.5777 : .1.5777 : .1.5777 : .1.5777 : .1.5777 : .1.5777 : .1.57777 : .1.5777 : .1.5777 : .1.5777 : .1.5777 : .1.57777 : .1.57777 : .1.57777 : .1.57777 : .1.57777 : .1.57777 : .1.577777 : .1.57777 : .1.57777 : .1.577777 : .1.57777 : .1.57777777777 : .1.577777777777777777777777777777777777	2		120	16	16	6		8,400 10,380 12,290	.0024 .0049	23,660 27,100 32,040	Dial		1	Minimum Average Maximum	6,150 9,000	.0027 .0033 .0042		604.	2,192	1.422	1.517	2-1/2	
		2	120		8	=	Mariana Mariana	9,060 11,400 12,840	00 ¹¹ 00	22,120 24,960 28,180	0460	724.	1	Minimum Average Mexturum		-0027 -0033 -0042	15.730 17.870 21.120	604.	2,192				#

Moolumm g divided by columm 10. Ecolumn 15 divided by columm 11. Loolumm 8 divided by column 15. Column 10 divided by column 17.

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Toble 1].---Dals-searing strongth and stitusion for compres C under tamelle lowding at 45m

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8		total rieste	<pre>:Total :Teated: For : for : teated: for : for : propor-: ulti-: : :tional : mete : : : limit : load :</pre>	Tor for ulti mete		Stress : Defor- 1	i Defor-	30mm	Bires: Defort Stress Defor- section section 1 1 1 1	<pre>iprapor-ifettedifettatu illoud int Ofint 90 illuit : illuit : illuit : illuit :</pre>	Touted	odifestod	Total	- 5			Average 1	5 5	Deformation 1 90° : A	Bereit	5	2 8	Li Averaça	There are a second seco	8	-9 -9	Average	congressive atre- Fropor-(Utimate tionel : limit-	tinate"	Rige : Learnards:	anrgin
(5)	(3) (4) :	1 (2)	: (5)	(9)	(1)	(8)	(6)	(00)	a	(12)	((1))	(41) 1	(14) : (15):	(16)	1 (11) 1	1 (38)	1 (61)	: (02)	(12)	(22)	(53)	(1)	(92)		(21) : (12)	(28) : (3) 1 (62)	1 (02)	(10)	(32)	(8)
: Jach						7777		4-1-2	प्राज्य ।				-1.77	12.2.1		Tar	P.4.4	Ant. zer I	Stath. P.a.L. P.a.L. (b) A car lach per lach per	ach per	ग्राम्य	La.1.	F.0.1		1.000 1.8-0	1 0001	1.000		1.9	1 3014 1	1 3
1/0	8	ж 	8		Marinem : 15,180 Merage : 20,080 Maximum : 22,580	16,180 20,080 22,580	10.00.00	10.01 1042 120.00 10.01 1001 1011 1011 1000 1001 1011 101	1 .0286 1 .0286	0.607	£	n	***	Mutauri 1	6,660 7,770 8,640	5.630	6.700	- 5400. - 5500.	: 5007 : 11007 : 11007	: (£007.0	1044 K	15,360 : 16,380 : 21,980 :	19,630	0.338	1.754	1.504 : 1.804 : 2	2,030 2	2-997 I	1.670 I	2-1/2 I.	******
	8	15	#		Manager 13, 330	11.3%		024,174 : 0600. 064,174 : 0600.	883	lz5-	67	2	29,	Ministra -	6,660 7.776 8.640	838	6. PUC :	1200	11200-	1 5600-	82. 18 11 18 11 18 11 18 11	15,960 1 14,880 1 21,980 1	19,620	1 866.	1.1754	1.96	2.020 2	2-476 1			1
1/2	132	2	8	6	Histows 12,220 1 Avarage 114,300 1 Maximus 115,660	12,230			0650	166	¥	Æ	112	Minimum 1 Averace 1 Maximum 1	5.18 5.55 5.55 5.55	999.9 992.9	0rti 9	0021 0036 1100	5600 1200	. 5500.	14,660 1 16,570 1 19,370 1	15,420	16,000	201	1 62.1	1,398	1.824 2	2.220 1		-1/2/1-2	
	130	97	ñ	9	i Marimum 11,950 Average 13,940 Marimum 15,600	11,980		.0084 : 23,530 : .0101 : 35,220 : .0123 : 25,450 :	0260 10 10 10 10	-336	8	ъ.	112	Average :	12.54 12.54	883	6,440	0027	0021 0022	- 95,00.	14,660	15,420 1	16.000 1	201	2,034 11	1,398 1 1,534 1, 1,958 1	1,524 2	2,165 2	I IR 2		3
1/1	8	8	8	я	Minimum 8,570 1 Average 11,570 1 Maximum 12,990 1	8,570 11,570 12,990		022.45 1 5200. 055.85 1 5200.	860 660 1	£01,	\$ 	ŝ	3,	Minteres 1 Averaço 1 Kesindo 1	6,660 1 7,770 8,610	1999 1999 1999	001.3	0025 0035 0045	1005) 1005)	ffco.	17.96 17.96	15,960 :	19.820 :	866.	1 442.1	1.201	2,020 1	1-727	1.16	2-1/2	
	8	115	21	•	1 Minimum 5,960 1 Average 10,140 1 Maximum 11,230	6,360 10,140 11,230		-0026 : 23,790 -0039 : 25,060 -0052 : 27,460	0380	\$0 1 -	67	2	5	Marneo 1	0110 012 010 010	1,500	6,700 1	1200 (£00	1200- 1200-	1 2600-	24.45	15,360 1	1 0511.61	856.	2.677 1	11.904 2.	2,020	1 (161	1.84	İ	s-1/2
2/12	130	1	11	9	Manage 11,270 Average 11,270 Maximum 12,660	9.720 11.270 12.660	\$\$900 \$600	8, % 8, %	516 2655 2655	9¥. 	8	₫,	112 :	Minimum : 5 Average : 7 Maximum : 6	5.960	209 209	6, MG	-002/ -0036 -0036	10021 2001- 2001-	1 9500-	19,566 19,570	12,090 1	16.000	102	1.75	1.13% 1.15%	1.824	1.730 1	1.760	5-1/2	
	130	\$	\$	•••••	Minimum : 9.210 1 Average : 10.570 1 Maximum : 13.050	9.210	1 6500. 1 9400.	99.98 19.98 19.98	-0530	Ltn.	8	đ.	III I	Matmum : 5,980 : Average : 7,310 : Maximum : 8,230 :	310	4, 450 5,580 5,440	6, WO 1	1283 1283	2400 5200 1200		19.370	12,420 1	16,000	1 201.	1. 749 1	1.336 1.824	1	1.688	1.628	11	3-1/2

Acolumn 8 titvided by column 10. Sociumn 19 divided by column 25. Acolumn 8 divided by column 19. Acolumn 10 divided by column 25.

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Table 14 .-- Bolt-bearing strength and critical dimensions for compres C under tensile loading at 90°

						8	Bolt bearing	50							WAY AB & TIMAA				holt-bearing	ne strass	::holt-bearing stress: Critical dimensions	dimensions
bolt	Mominal Nominal: Fanel ::	Fanel :	: Number of u	of speci	T BUSH	a 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Proportional 11mit	tonal	Ultimate	ste	Ratio ::	Wumber	:: Number: Variation:	Proportional limit		: Bat: Ultimate: of	Batio : of :	Batio : Modulus:: of : of ::	:: tompressive	re stress:	Edge	End
101			Total : teated:	n has	Tested: for : ulti-: mate : losd :		Stress	Defor-	24148 24148 24148	Defor- mation	<pre>:propor-:: of :tional ::speci- :limit :: mens : to wir:: timate*:;</pre>	speci-		Streas : Defor-	Defor-	8t7688	stress :propor-: eles- :tional :ticity : 11mit : : to ul ² : :timate ² :	elas- ticity	tional:	11 timete	:: Propos-:Ultimate ¹ : clearance	nergin :
(B)	: (2) :	(2)	(#) ::	(2)	(9)	: (L)	(2)	(6)	(01)	(11)	(12)	:: (13)	(††) :	(15)	(91)	(11)	(18)	:: (61)	: (02)	(12)	: (22)	: (23)
Inch	Inch				 		P. 0.1.	Inch	P. s. 1.	Inch				P. B. 1.	LIDCD DEF	P.a.1.		1.000 -			: Bolt diameters	Bolt
		9		8	8	Minimum : Average : Maximum :	12,950 :	0.0039	29,860 32,360 36,520	0650.0	0.5 ⁴ 1 :	8	: Minimum : Average : Moximum	5,240	0.0028	15,880 19,060 21,180	0.315	1.766 :: 1.945 :: 2.132 ::	2.917 :	1.696	: 3-1/2	
-	: 1/4 : :	20 11	1 72	1 61	10	Minimum : Average : Maximum :	13,740	.0065	25,930 32,590 37,280	Dial	££6.	4	mumitiki	4, 620 5, 500 6, 010	4500. 1029	18, 340 19,660 22,080	, 280	1,698 :: 1,884 :: 2,017 ::	3.218	1.658		#
±/₽		240	1 9T	16 1	12	Minimum : Average : Maximum :	12,100 114,070 1	0000-	31,950	0545	.382	÷.	i Minimum i Average Maximum	4, 480 5, 390 6, 600	-0023 -0023	15,570	346.	1.532	2.610	2.364	#	
	- 1/2	140	3	2	97	Minimum : Average : Mailmum :	11,860 13,710 13,710 17,890	.0105	32,860 35,790 43,720	.0680 .0784	.383	Éŧ	Minimum Average 1 Maximum	4,480 6,530 6,600	.0027 .0033	15,570	¥.	1.540 11	2.5Ht	2.299		#
		10	1 11	191	16 1	Minimum Averaçe Maximum	7,870 10,960 13,660 1	.0028 .0045	20, 120 24, 250 27, 340	0140. 2250.	154.	8	: Minimum : Average : Maximum	5, 240 6,000 7, 250	.0028	15,880 19,060 21,180	.315	1.766	1.827	1.274	5-1/2	
	τ	14C 11	5	5	8	Minimum Averaça Maximum	9,160 11,130 14,010	.0025 .0043	21,840	0380	-50g-	8	i Minimum Average I Maximum	4,590 5,470 6,580	.0025 .0029	16.610 19,020 21,130	. 265	1,631 :: 1,850 :: 2,061 ::	2.035	1.151	2-1/2	
1/2		3		57	8	Minimum Average Maximum	9,040 11,090 13,460	-0032 -0043	19,590 22,640 26,510	Diel	11 06h.	58	Minimum Average Maximum	4,590 5,470 6,580	.0025 .0029	16,610 19,020 21,130	.288	1,631 11 1,660 11 2,061 11	1 L2072	190		3-1/2
		140	11	15	10	Minimum : 10.050 Average : 11.440 Maximum : 13.650	10.050 11.140 13.650	.0055	19.660 25.410 30.300	Dial removed	0 <u>6</u> 4.	143 143	Minimum Average Maximum	4, 480 5, 390 6, 600	.0033 .0033	13,820 15,570 19,360	346.	1.322 :: 1.640 :: 1.834 ::	2.122 1	1.632	#	
	2 7 		41	4	60	Minimum 9, Average : 11, Maximum : 13,	9,500 11,440	.0052 .0064	26,080 27,530 31,100	Dial	914.	43	: Minimum : Average : Maximum	4, 480 5, 390 6, 600	.0027 .0033	13.820 15.570 19.360	· 346	1.322 ::	2.122	1.768		

l logiumm 8 divided by column 10. 2001umm 5 divided by column 17. Logiumm 5 divided by column 15. 201umn 10 divided by column 17.

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compressionBalge clearanceRad marginEdge clearanceRad marginRader tunderUnder tuneUnder	Diam-	Diam-:Thick- : Grain ater :ness of: angle	Grain		FPL compreg-	regl		Col	Commercial	compreg A	Commercial	compreg	8	Con	Commercial	compreg C	
Inder Under Ioeding sive loading sive loading sive loading sive loading sive loading sive loading loading loading loading sive loading loading <thloading< th=""> <thloading< th=""> <thloadin< th=""><th></th><th>:Serqmon</th><th>of face.</th><th>Edge cl</th><th>esrance :</th><th></th><th>argin :</th><th>Edge cl.</th><th>sarance :</th><th>End margin</th><th>Edge clearance</th><th></th><th>argin :</th><th>Edge clearance</th><th>arance :</th><th>am bra</th><th>margin</th></thloadin<></thloading<></thloading<>		:Serqmon	of face.	Edge cl	esrance :		argin :	Edge cl.	sarance :	End margin	Edge clearance		argin :	Edge clearance	arance :	am bra	margin
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$			0 10 10 10 10 10 10 10 10 10 10 10 10 10	Under compres- sive loading	Under tensile	Under modified compres- sive loading	Under tensile losding		Under tensile loading	Under : Under moüffled:teneile compres-losding sive : loading :	Under Under Compres- tenst sive loading	r Under le modified 1g:compres- : sive :loeding	. Under : tensile: :losding:	Under : compres-: sive : loading :	Under tensile: loading:	Under Under tensile modified loading compres- sive loading :	Under tensile loading
Inch Degrees Bolt di-isolt di-golt di-golt di eneres $1/4$ 0 $1-1/2$ $2-1/2$ 4 $1/4$ 0 $1-1/2$ $2-1/2$ 4 $1/2$ 0 $1-1/2$ $2-1/2$ 4 $1/2$ 0 $1-1/2$ $2-1/2$ 4 $1/2$ $1/2$ $2-1/2$ 4 4 $1/2$ $1-1/2$ $2-1/2$ 3 4 0 $1-1/2$ $2-1/2$ 3 4 $1/4$ $1/5$ 2 $2-1/2$ 3 4 $1/4$ $1/2$ $2-1/2$ 4 $-1/2$ 3 4 $1/4$ $1-1/2$ 2 $2-1/2$ 4 $-1/2$ 3 $1/2$ 0 $1-1/2$ 2 $2-1/2$ 4 $-1/2$ $1/2$ 0 2 $2-1/2$ 3 3 $-1/2$ 3 $-1/2$ 3 $-1/2$ 3 $-1/2$ <th>b</th> <th>(2)</th> <th>(2)</th> <th>(†)</th> <th>: (2) :</th> <th>(9)</th> <th>: (1)</th> <th>(8)</th> <th>(6)</th> <th>ļ</th> <th><u> </u></th> <th><u> </u></th> <th>. (15) :</th> <th>(16)</th> <th>(11)</th> <th>(18)</th> <th>(61)</th>	b	(2)	(2)	(†)	: (2) :	(9)	: (1)	(8)	(6)	ļ	<u> </u>	<u> </u>	. (15) :	(16)	(11)	(18)	(61)
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Inch	1	Degrees	Bolt di-	Boltdi-		Boltdi-	Bolt di- ameters	Boltd1-	Bolt di- Bolt di- ameters : ameters	Bolt di- Bolt d ameters : ametel	i-:Bolt di-	:Boltd1-:	Bolt di-	Boltdi-	:Boltdi-:Bolt di-:	Boltd1-
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	ę,	1/4	ဝင်္ကဝိ	1-1/2 2 1-1/2	: 2-1/2 : : 2-1/2 : : 3-1/2 :	4 P	4 MM	1-1/2 1-1/2 1-1/2	3 2-1/2 3-1/2	њюм 1 н. и м.				 ພ.ພ.ພ	3-1/2 : 2-1/2 : 3-1/2 :		t m t
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	* -	1/2	ంసేరి	2 2 1-1/2	3 2-1/2 4	a t	in mat	ณณณ	2-1/2 2-1/2 4-1/2					1-1/2 2 2	5-1/2 5-1/2		# m#
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		1/h	070	1-1/2 2 1-1/2	2 2-1/2 3	-	3-1/2 2-1/2 4	1-1/4 1-1/2 1-1/4	4 2-1/4	2-1/2 2 1				1-1/2 2 1-1/2	3-1/2 2-1/2 2-1/2		4-1/2 2-1/2 3-1/2
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	1/2	1	076	1-3/4 2 1-3/4	2-1/2 2-1/2	3	3-1/2	1-1/4 1-1/2 1-1/4						1-1/2 1-1/2 1-1/2	2-1/2 2-1/2		4 2-1/2 4-1/2
$\begin{array}{cccccccccccccccccccccccccccccccccccc$			0 7 8	<u>ุ</u> ญ ญญญ		MMM		1-3/4 2 1-3/4									
1-1/2 3 1-1/4 2-1/2 2 2 1-1/2 2-1/2 2 1-1/2 2-1/2 2-1/2 2 1-1/2 2-1/2 2-1/2	,	1/2	ంసేరి	1-1/2 2 1-1/2		2-1/2 3 3		1-1/4 1-1/2 1-1/4		2-1/2 2 2-1/2				1-1/2 2 1-3/4		2-1/2 2-1/2 2-1/2	
			040	1-1/2 2 2		3 2-1/2 3		1-1/4 1-1/2 1-1/2									

Table 15 .-- Summary of critical dimensions

Avalues taken from table 6 of Report No. 1523A.

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Table 16 .-- Summary of bearing stresses

of :c	and in some of						and a second sec			A REAL PROPERTY OF A REAL PROPER			0	22		Serdinon Toto Forman		د
• •	. Sardmon	of face: ply	compregiof face:At proportional : ply : limit	rtional : it :	At ul	timate :	At proportional	ttional : It :	At ult:	ultimete	At proportional	rtional : it :	At ultimate		At proportional	rtional : it :	At ultimate	imate
••••••			: Under :compres- : sive :losding2	: Under : Under : Under :compres-:tenalle:compres : sive :loading: sive :loading	Under : s:compres-: s: sive : :loading2:	Under tensile	Under compres- sive loading2	: Under : Under :tensile:compre: :loading: sive :loading:	Under : compres-: sive : loading2	Under : tensile	Under : Compres-:t sive:1 loading2:	: Under : tensile: losding:	: Under : : compres- : sive : :loading2:	Under : tensile: loading:	Under : Compres- : sive : loading2	: Under : tensile: loading:	Under : compres-: sive : loading2:	: Under : tensile : loading
(1)	(2)	(3)	(†)	(2)	(9)	(2)	(8)	(6)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)
	Inch :	Degrees: P.s.1.	P. s. 1.	P. 8.1.	P. 8. 1.	P. s. 1.	P.s.1.	P. 8. 1.	P. 8. 1.	P. 8. 1.	P. e. 1.	P. s. 1.	P. s. 1.	P. 8. 1.	P. e. i.	P. s. 1.	P. 8.1.	P. 8.1.
	. #//T	0£8	22,800 21,390 24,430	20,100: 20,430: 17,810:	34,810 30,730 31,340	33,530: 32,840: 32,780:	14,870 16,500 15,490	13.150: 14,240: 13.510:	21.930 26.050 24.300	28,070: 27,460:. 27,120:.	18.040		28,690		18,350 : 17,240 : 17,840 :	: 17,310: : 18,340: : 17,600:	32.890 30.050 30.710	: 32,330 32,480
;	1/2	90 90	20,690 22,250 24,330	15,670: 16,330: 15,630:	34, 240 34, 060 34, 710	35, 350 : 36, 300 : 39, 420 :	18,370 18,980 17,820	12,830 13,160 13,990:	29,890 29,990 32,020	30,700 31,990 32,270	22,600		38,100		19,740 16,570	15,420: 14,120: 13,890:	36,240	36, 240 35, 620
	1/4	90 90	16,130 18.500 17,630	: 14, 630: 26, 910 : 14, 780: 28, 360 : 12, 960: 31, 470	26.910 28.360 31.470	28,360: 28,120 27,610:	11,770 13,030 11,590	10,160: 9,540: 10,050:	19,510 22,090 19,630	21, 230: 21, 860 21, 280	11,990		24,140		14,250	10,720: 10,860: 11,060:	27,240 27,790 27,980	22,940
1/2 :	1/2	o 7 06	16,980 19,940 19,580	: 15,780: 27,670 : 13,880: 30,660 : 14,190: 30,660	27,670 30,660 30,660	30,450 29,700 30,000	11,620 14,460 13,550	9,900: 11,220: 10,280:	21,850 25,340 26,200	21.930: 25,380: 22,720:	16,650		29,780		16,370 13,320 13,050	10,890 11,070 11,140	29.180. 25,900 24,870	26.030 27.100 26.470
	-1	0 2 2 0 2 0 2 0	15,740 17,850 18,010		29,120 34,080 32,560		14,420 16,240 15,900		27,700 28,370 27,660		15,040		36,130		14,410		36, 240	
	1/2	90 ⁴⁵ 0	14,890 15,910 15,040		27.730 29.380 27.610		11,180 11,110 10,020		21,860 21,600 21,750		13,070		24, 440		12,120 : 11,740 : 10,540 :		25,560 27,160 23,570	
 4		o 1, 8	14.700 16.160 13.960		27,160 29,120 31,520		10,050 : 11,030 : 10,970 :		21,600 23,170 22,740		13,400		28, 520		13,400		28,230	

4 Values taken from table 7 of Report No. 1523A. ² Includes modified compressive loading.

Z M 65581 F

Table 17 .-- Summary of deformations

"egiof face: At proportional At ulti t "ply Under Under Under Under Compres- "under Under Under Under Compres- "sive loading: "isoding: ioading: (j) (u) (j) (u) (j) (i) Degrees Inch 100070 0.0070 0 0.0070 0 0.0070 0 0.0079 0 0.0079 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 00071 0 0071 0 0071 0 0071 0 0071 0 0071 0 0072	1.2.2							Contraction of the second s			
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$		proportional : limit :	At ultimate		At proportional limit		At ultimate :	At proport: 11mit	proportional : limit :	At ultimate	mate
(2) (3) (4) (5) (6) Inch Degrees Inch Inch Inch Inch 1/4 45 0 0.0070 0.0057 0.0277 0 1/2 45 0.0102 0.0057 0.0276 0 0 1/2 45 0.0102 0.0057 0.0276 0 0 1/2 45 0.0102 0.0057 0.0266 0 0 1/4 45 0.0106 0.0057 0.0260 0 0 0 1/4 45 0.0106 0.0057 0.0260 0 </th <th>Under : Under : : : : : : : : : : : : : : : : : : :</th> <th>Under : -: tensile: :loading:</th> <th>Under : U compres-:te sive :lo coding2:</th> <th>: Under : Und tensile:comp :loading: si :loading: si</th> <th>: Under : Under compres-:tensile sive :loading</th> <th>: Under e:compres- g: sive :loading2</th> <th></th> <th>: Under : compres-: sive : loading2:</th> <th>Under tensile loading</th> <th></th> <th>Under tensile loading</th>	Under : : : : : : : : : : : : : : : : : : :	Under : -: tensile: :loading:	Under : U compres-:te sive :lo coding2:	: Under : Und tensile:comp :loading: si :loading: si	: Under : Under compres-:tensile sive :loading	: Under e:compres- g: sive :loading2		: Under : compres-: sive : loading2:	Under tensile loading		Under tensile loading
Inch Degrees Inch	(2) : (2)	(6)	(10)	(11)	(12) : (13)	(11)	: (12) :	(16)	(11)	(18)	(61)
1/4 0 0.0070 0.0059 0.0257 0 1/4 45 .0085 .0067 .0276 .0276 1/2 45 .0085 .0057 .0256 .0259 1/2 45 .0102 .0082 .0259 .0256 1/4 45 .0076 .0037 .0280 .0390 1/4 45 .0076 .0037 .0280 .0300 1/2 45 .0076 .0037 .0280 .0322 1/2 45 .0070 .0034 .0300 .0300 1/2 45 .0012 .0034 .0300 .0300 1 45 .0012 .0034 .0300 .0300 1 45 .0102 .0034 .0300 .0300 1 45 .0102 .0049 .0300 .0300 1 45 .0122 .0049 .0300 .0300 1 45 .0128	Inch : Inch	Inch :	Inch I	Inch : In	Inch : Inch	-: Inch	Inch	Inch :	<u>Inch</u> :	Inch :	Inch
1/2 0 .0079 .0082 .0350 1/2 145 .0102 .0089 .0390 1/4 145 .0106 .0081 .0280 1/4 145 .0076 .0037 .0280 1/4 145 .0076 .0037 .0280 1/2 145 .0076 .0037 .0259 90 .0070 .0037 .0259 90 .0070 .0037 .0300 90 .0070 .0047 .0352 90 .0070 .0055 .0352 1/2 145 .0102 .0300 1 145 .0102 .0047 .0300 1 90 .0128 .0128 .0312 .0312	0.0224: 0.0096 .0260: .0085 .0269: .0087	0.0058 .0063	0.0294 : 0 .0331 : .0266 :	0.0440: 0.0 .0361:	0.0097			0.0077 : : 0085 : : 0079 :	0.0076: -0073: -0071:	0.0427 : -0453 : -0389 :	0.0402 .0286 .0286
1/4 145 .0076 .0071 .0250 2070 .0074 .0259 1/2 145 .0076 .0074 .0300 2055 .0328 1/2 145 .0102 .0055 .0328 2012 .0055 .0328 0352 .0352 .0352 .0471 .0352 .0471 .0352 .0352 .0471 .0352 .0352 .0352 .0471 .0352 .0352 .0352 .0352 .0352 .0352 .0352 .0352 .035552 .0355552 .035552 .035552 .0355552 .0355552 .0355552 .0355552 .035555552 .03555555555555555555555555555555555555	.0420: .0127 .0450: .0138 .0427: .0093	-0096 -0096 -0086	-0527 -0552 -0453	.0558: .0 .0558:	0112	0572		. 0092 . 0092 .	:0010.	.0439 : .0645 :	
1/2 45 .0061 .0055 .0328 90 .0087 .0053 .0352 1 45 .0102 .0053 .0352 .0300 .0367 .0471 1 45 .0122 .0122 .0411 .0411 90 .0128 .0128	.0191: .0065 .0247: .0065 .0221: .0065	. 0038: .0041	.0308 .0499 .0278	0322 .0	0076	2 T-50		: 1200. : 1300.	- 0000- - 00100-	-0369 -0468 -0340	
0 .01020471	.0287: .0071 .0277: .0078 .0233: .0078	.0059: .0059:		0387: 0		+0564		-0102 : -0105 : -0103 :		-0340 -0550 -0472	.0527 .0537
. 00086	0150		.0746 .0672 .0660	0		0480.		-0153		.0868	
. 1/c . 4000980326			.0565 .0614 .0565	0.	6600	.0402		- 0095 - 0095 - 0098		1450- 1420-	
1 45 .0117 .0450 .0188			-0735 -0755	0	0122	+0702		.0121			

Z M 65582 F

of :	compreg	: angle : : of : face :	: com :	preg	: compre	∍g ^u A ^u	: compr :		: compr	eg "C"	:Average : for : comme : comp	all rcial	: for thick	all nesses ²
			Under com- pres- sive load-	Under : ten- : sile : load- : ing	Under : com- : pres- : sive : load-	Under : ten- : sile : load- : ing	:Under : com- : pres- : sive : load-	:Under : ten- : sile : load- : ing	:Under : com- : pres- : sive : load-	:Under : ten- : sile : load- : ing	:Under : com- : pres- : sive : load- : ing	:Under : ten- : sile : load- : ing	:Under : com- : pres- : sive : load-	:Under : ten- : sile : load- : ing
Inch	Station in case of	Degrees:	L			:	:	:	:	: *	:	:	: :	:
1/4	1/4	: 45 : : 90 :	•70 •78	•62 •54	0.68 .63 .64	0.47 52 50	: 0.63	1 • • • • • • • • • • • • • • • • • • •	• 0.56 • 57	• 0.54 • 57	: : • • • • • • • • • • • • • • • • • •	: :	: : :	******
-/ •	1/2	0 45 90	.60 .65 .70	.44 .45 .40	.61 .63 .56	.42 .41 .43	•59 ••••	: • • • • • • • • • • • • • • • • • • •	•54 •47 ••52	: .43 : .40 : .38	:	: • • • • • • • • • • • • • • • • • • •	: • • • • • • • • • • • • • • • • • • •	: • • • • • • • • • • • • • • • • • • •
:	1/4	0 45 90	.60 .65 .56	•52 •53 •47	•60 •59 •59	•48 •44 •47	•50	:	•52 •49 •53	· .45 · .41 · .48	: : • • • • • • • • • : • • • • • • • • •	: • • • • • • • • • • • • • • • • • • •		: • • • • • • • • • • • • • • • • • • •
1/2	1/2	45 90	.61 .65 .64	•52 •47 •47	•53 •57 •52	.45 .44 .45	• • 56	: • • • • • • • • • • • • • • • • • • •	. 56 .51 .52	: .42 : .41 : .43	: • • • • • • • • • • • • • • • • • • •	: • • • • • • • • • • • • • • • • • • •		: : :
; ; ; ;	1	0 : 45 : 90 :	•54 •52 •55	• • • • • • • • • • • • • • • • • • •	•52 •57 •57	•••••	.42	1 • • • • • • • • • • • • • • • • • • •	.40	:		: :	· · · · · · · · · · · · · · · · · · ·	: : :
1	1/2	0 : 45 : 90 :	-54 -54 -54	•••••	-51 -51 -46	•••••	•53		47 43 45	: • • • • • • • • • • • • • • • • • • •			•••••	: :
: : ;	1 :	45 :	•55 •44	•••••	.48 .48			: • • • • • • • • • • •	• • • • • • • • •	: • • • • • • • • • •		: • • • • • • • : : • • • • • • • :	•••••	: :
:	:	:	:	: vA				; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;	rienta			: :		
1/4	1/2	* 90 : 0,45,: * 90 :	.65	•59 •43	•65 •60	.50 .42	•63 •59	: :	•57 •51	•55 •40	. 0.62 3 _{.57}	3.41	0.62	0.52
:	1/4 1/2 1	0, 45, & 90 0, 45, & 90 0, 45, 0, 45,	•60 •63 •54	.51 .49	•59 •54 •55	.46 .45	•50 •56 •42	:	•51 •53 •40	· .45 · .42	•53 •54 3.46	. "46 . "44	•54	
1	1/2 1	.0, 45, :	•54 •51	• • • • • • •	•49		•53 • • •47	: • • • • • • • • • • •	•45	:	: •49 :		l Jigt	:

 $\frac{1}{Values}$ taken from tables 2 through 5 of Report No. 1523-A.

2-For commercial compregs only.

ZBearing length equals twice the diameter of the bolt. Ratio shown is not included in average for each diameter of bolt.

2 M 65583 F

Table 19.--Summary of ratios of bolt-bearing stress to compressive stress $^{\rm L}$

-un-	Dlam-: Thick- : Grain	: Grain		FPL compres	reg		Co	COMMBICIAL	compreg A	4	8	Commercial	compreg B	<i>m</i>	8	Commercial	compres	c
of .	Serquos	of face ply	compress of face At proportional	rtional : it :	At ult	imate	At proport limit	proportional : limit	At ultimate		At proporti	proportional : limit :	At ultimate		At proporti	lano	At ultimate	imate
		peting: speci- : mens :	Under compres- sive loading	101 00.01 0.01 0.01 0.01 0.01 0.01 0.01	Under Compres- sive loading		Under compres- sive loading	Under : Under : Under : Under : Under : tensile :compres- tensile :compres-:tensile :compres- loading: sive :loading: sive : :loading : :loading :	Under compres- sive loading		Under compres- sive loeding	: Under : Under : tensile: compre : loading: sive : loading:	Under compres- sive	Under : tensile: loading:	Under: Under: tensile:compres- loading: sive : loading :	: Under : tensile: losding:	Under Under tensile:compres- loading: sive :loading	: Under tensile loading
(1)	(2)	(3)	(#)	(5)	(9)	: (1) :	(8)	(6)	(10)	(11)	(12)	: (13)	(14)	(12)	(16)	(11)	(18)	(19)
Inch :	Inch	.Degrees:																
	1/h	0£8	2.31 2.24	1.17	1.44	1.35 1.43 1.27	2.52 2.52 2.65	2.26 2.22 2.33	1.42 1.61 1.66	1.71 1.77	2.68		1.56		2.54 2.60 2.97	2.84 2.74	1.76 1.59 1.61	1.58 1.63
t 5	1/2	90 th o 90	2.26 2.19 2.24	£55€ 1111	1.67 1.45 1.52	1.63 1.66 1.84	2.66 2.83 3.51	1.86 2.11 2.63	1.98 1.93 2.01	1.94 1.91 2.03	3.34		2.05		3.29 2.77 2.82	2.11	2.24 2.17 1.99	2.23
	1/1	90 250 90	1.64	1.24 1.25	1.11	1.14 1.23 1.07	1.85 1.99 1.98	1.52 1.52	1.27 1.37 1.34	1.32 1.32	1.78		1.31		1.97 2.04 2.49	1.76 1.62 1.96	1.45	1.15
1/2 :	1/2	0 [90	1.69 1.96 1.87	1.35 1.35	1.35	1-33 1-36 1.46	1.68 2.15 2.67	1.60 1.91	1.63 1.63	1-	2.46		1.61		2.23	1.72	1.52 1.52	02-1 91-1
		90.50	1.96 1.75 1.97		1.42		2.33 2.71 2.66		1.94 1.80 1.84		2.36		1.61		2.14		1.86	
	1/2	0 . 78	1.42		1.28 1.22 1.20		1.63 1.88 1.97		1134		1.93		1.32		1.89 1.93 1.84		1.65	
 -		၀ႜႜၟ႙	1.83 1.58 1.37		1.33 1.27 1.37		1.60 1.79		1.50		2.10		1.43		2.27		3.45	

Zvalues taken from tables 2 through 5 of Report No. 1523A.

Z M 65584 F

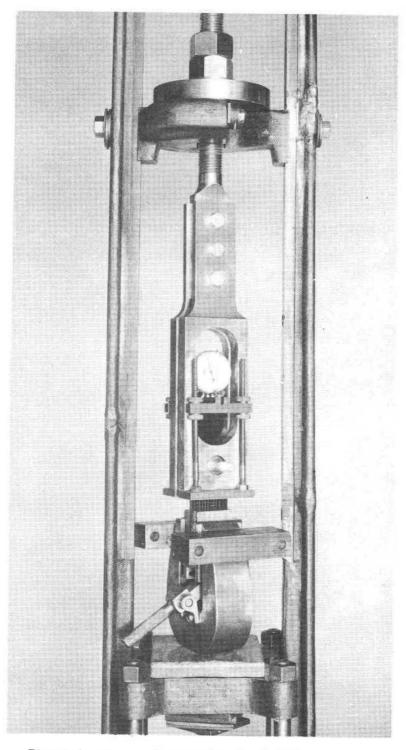


Figure 1.--Improved apparatus for bolt-bearing tests under tensile loading.

2 M 66507 F

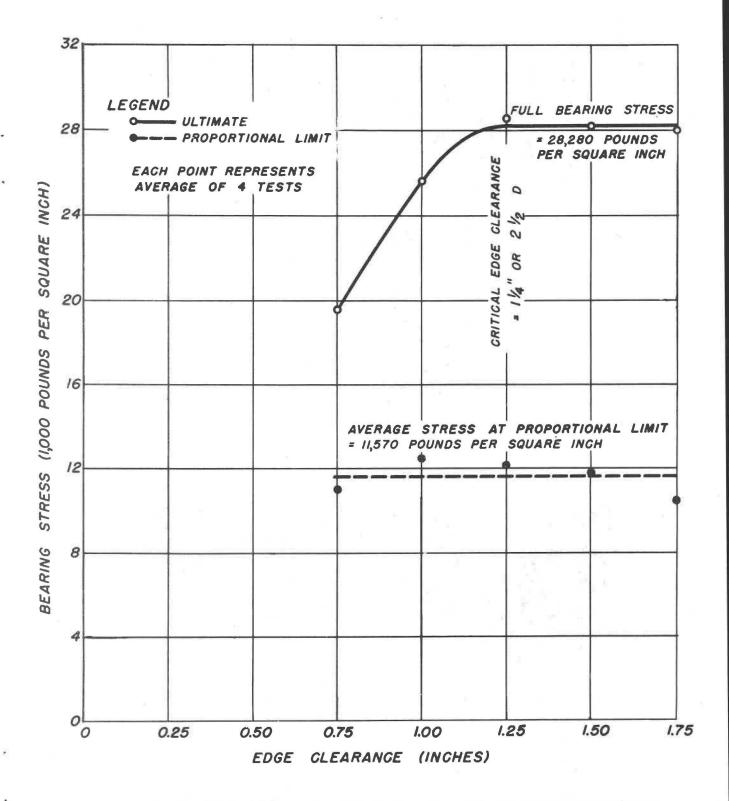
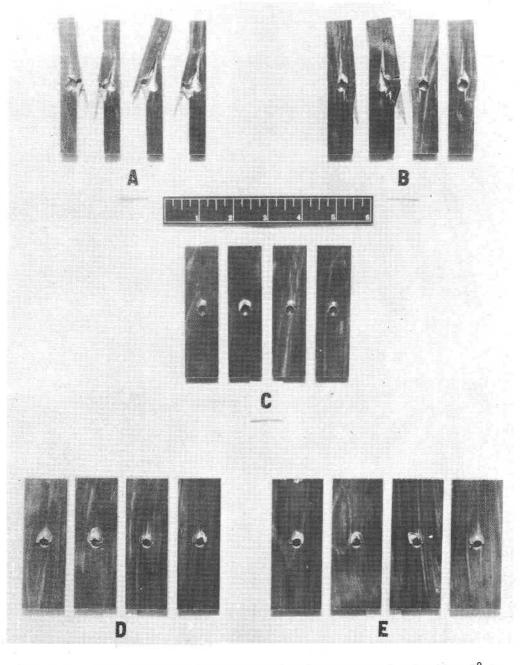
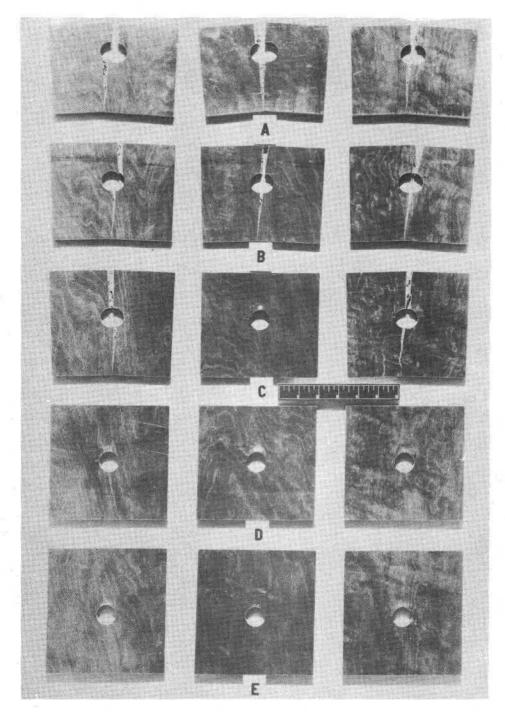


Figure 2.--The determination of critical edge clearance and average proportional limit and ultimate bearing stresses for a 1/2-inch bolt in 1/4-inch compreg C under tensile loading at 45° to the grain of the face plies.

Z M 65585 F

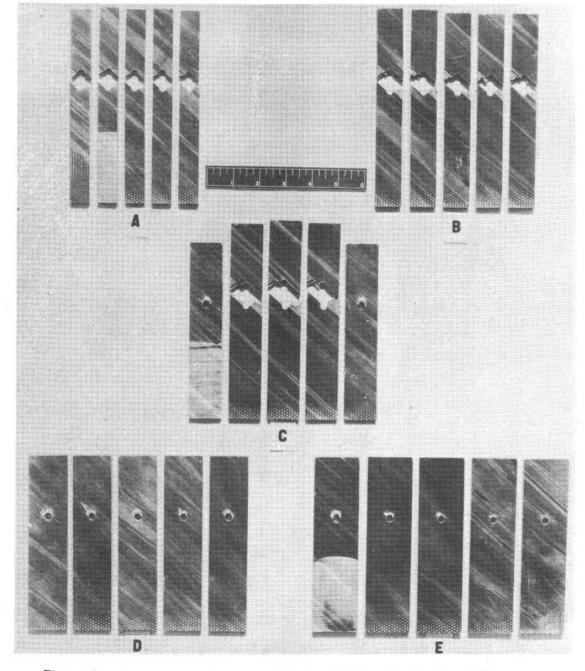


- Figure 3.--Edge-clearance series tested under compressive loading 0° to face grain. One-quarter inch bolt in 1/4-inch commercial, cross-banded, hard maple compreg C.
 - A. Edge clearance = 1/4 inch; average proportional limit load = 1,225 pounds; average ultimate load = 1,786 pounds.
 - B. Edge clearance = 3/8 inch; average proportional limit load = 1,225 pounds; average ultimate load = 1,945 pounds.
 - C. Edge clearance = 1/2 inch; average proportional limit load = 1,188 pounds; average ultimate load = 2,206 pounds.
 - D. Edge clearance = 5/8 inch; average proportional limit load = 1,225 pounds; average ultimate load = 2,234 pounds.
 - E. Edge clearance = 3/4 inch; average proportional limit load = 1,262 pounds; average ultimate load = 2,158 pounds.



- Figure 4.--End-margin series tested under modified compressive loading 0° to face grain. One-inch bolt in 1/2-inch commercial cross-banded hard maple compreg C.
 - A. End margin = 1-1/2 inches; average proportional limit load = 6,400 pounds; average ultimate load = 8,200 pounds.
 - B. End margin = 2 inches; average proportional limit load = 7,200 pounds; average ultimate load = 11,940 pounds.
 - C. End margin = 2-1/2 inches; average proportional limit load = 7,667 pounds; average ultimate load = 15;333 pounds.
 - D. End margin = 3 inches; average proportional limit load = 7,300 pounds; average ultimate load = 15,360 pounds.
 - E. End margin = 3-1/2 inches; average proportional limit load = 7,900 pounds; average ultimate load = 15,540 pounds.

Z M 66511 F



- Figure 5.--Edge-clearance series tested under tensile loading 45° to face grain. One-quarter-inch bolt in 1/4-inch commercial cross-banded hard maple compreg C.
 - A. Edge clearance = 3/8 inch; average proportional limit load = 1,100 pounds; average ultimate load = 1,328 pounds.
 - B. Edge clearance = 1/2 inch; average proportional limit load = 1,365 pounds; average ultimate load = 1,817 pounds.
 - C. Edge clearance = 5/8 inch; average proportional limit load = 1,290 pounds; average ultimate load = 2,103 pounds.
 - D. Edge clearance = 3/4 inch; average proportional limit load = 1,235 pounds; average ultimate load = 2,088 pounds.
 - E. Edge clearance = 7/8 inch; average proportional limit load = 1,245 pounds; average ultimate load = 2,169 pounds.

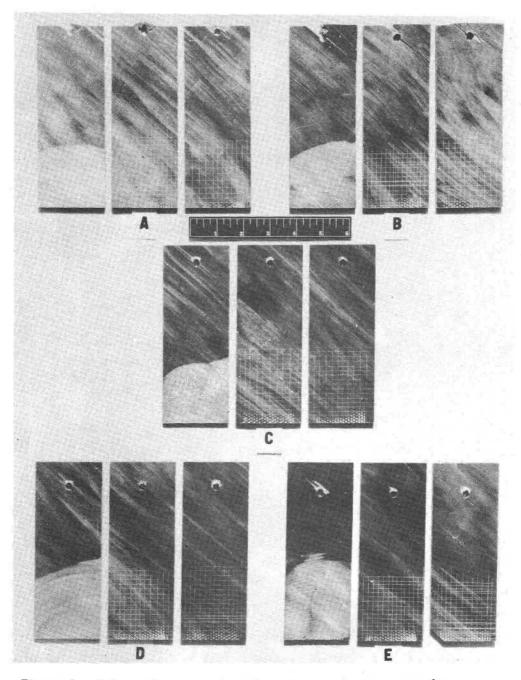
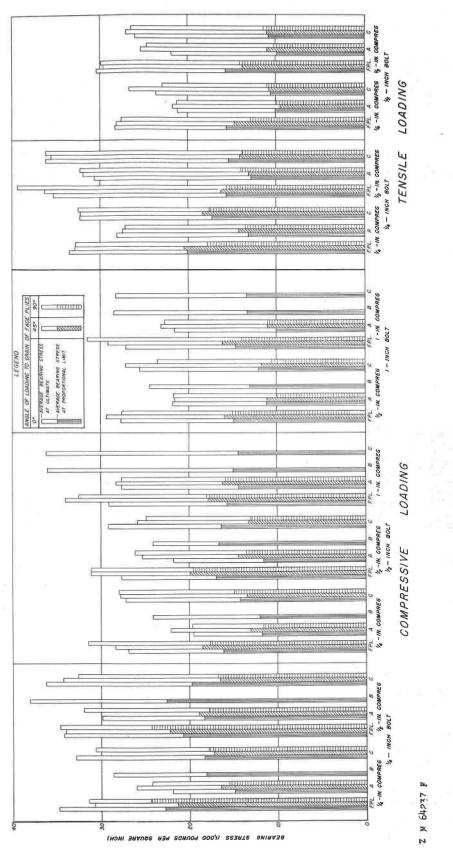
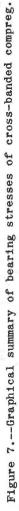


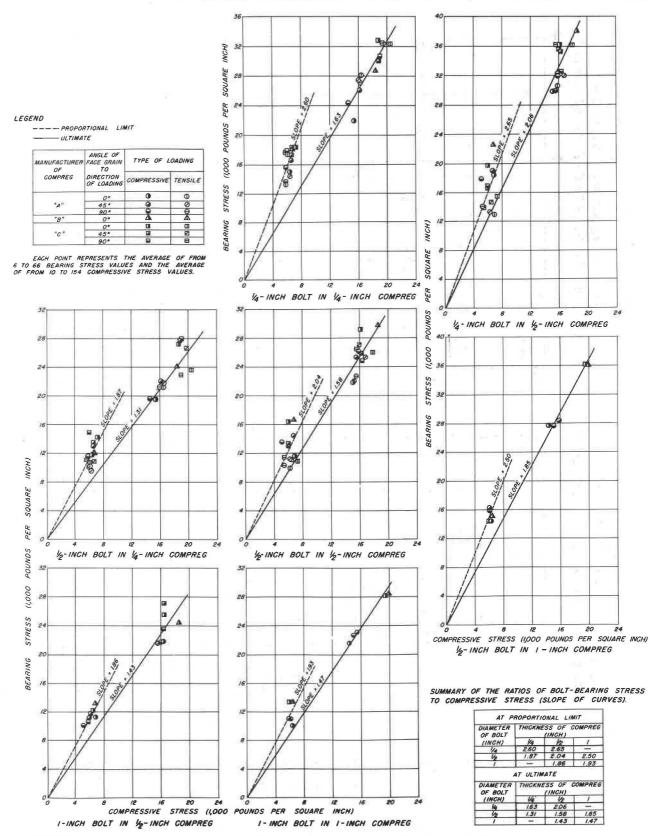
Figure 6.--End-margin series tested under tensile loading 45° to face grain. One-quarter-inch bolt in 1/4-inch commercial cross-banded hard maple compres C.

- A. End margin = 1/4 inch. Maximum load attained, 738 pounds, was less than average proportional limit load.
- B. End margin = 1/2 inch; average proportional limit load = 1,117 pounds; average ultimate load = 1,755 pounds.
- C. End margin = 3/4 inch; average proportional limit load = 1,067 pounds; average ultimate load = 1,992 pounds.
- D. End margin = 1 inch; average proportional limit load = 1,025 pounds; average ultimate load = 2,161 pounds.
- E. End margin = 1-1/4 inches; average proportional limit load = 1,108 pounds; average ultimate load = 2,022 pounds.

Z M 66508 F







Z M 64238 F

Figure 8.--Relationship between bearing stress and compressive stress of commercial compregs A, B, and C.

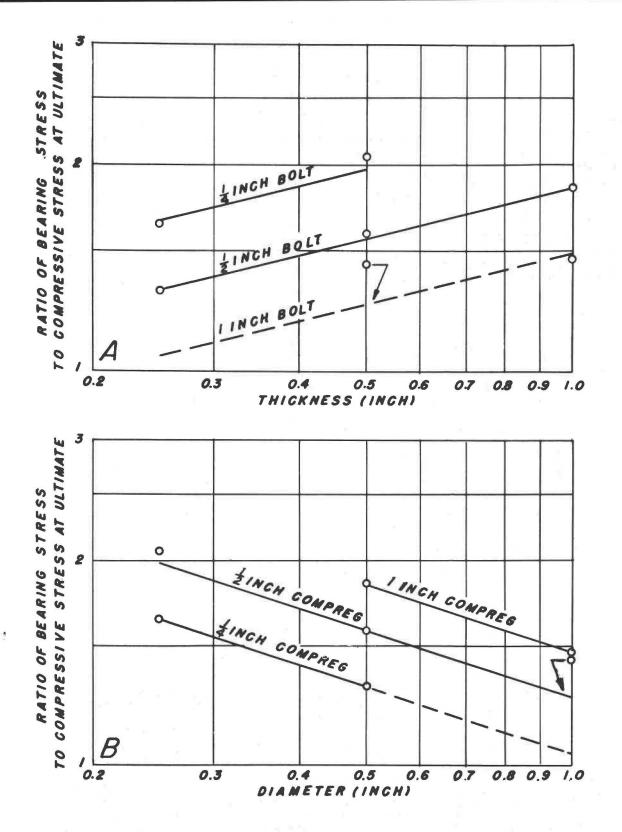


Figure 9.--Effect of variation of thickness (A) and diameter (B) on the ratio of bearing stress to compressive stress at ultimate. Circles are actual average ratios obtained from tests in this investigation. 2 M 65590 F

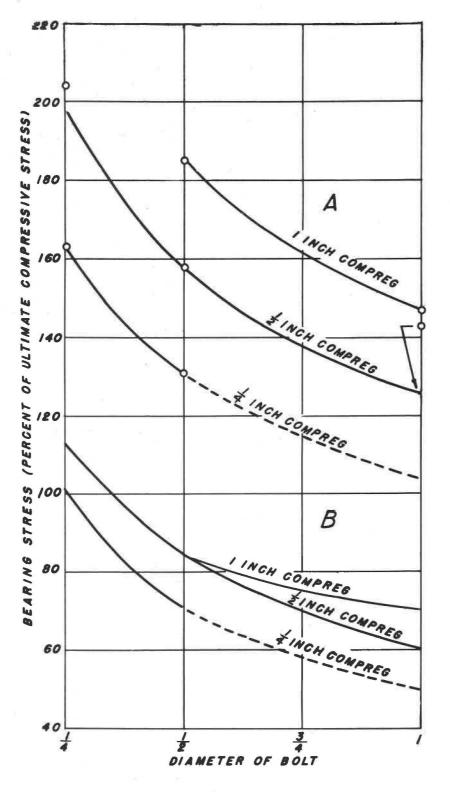
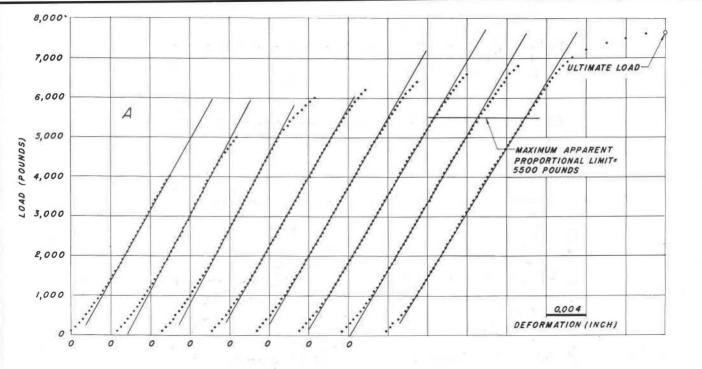


Figure 10.--Ultimate (A) and proportional limit (B) bearing stresses of commercial cross-banded compreg expressed as percent of ultimate compressive stress for three diameters of bolts (1/4, 1/2, and 1-inch) and for three thicknesses of compreg (1/4, 1/2, and 1-inch).



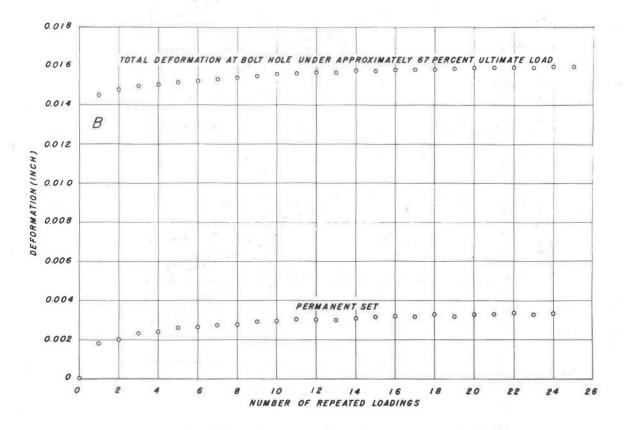


Figure 11.--Data from two repeated-loading bolt-bearing tests plotted on rectangular coordinates to show (A) selection of maximum apparent proportional limit and (B) verification of selected maximum apparent proportional limit by 24 repeated loads showing little increase in total deformation or permanent set.

Z M 65592 F